- ① SEE CROSS SECTIONS FOR SLOPES OUTSIDE THE LIMITS OF THE SHOULDER.
- ② CONSTRUCT TURF REINFORCEMENT MAT 4 WITHIN THE LIMITS SHOWN ON THE TYPICAL SECTION, AND BETWEEN THE STATIONS SHOWN ON THE PLAN SHEETS, TO PROTECT THE LEFT ROADWAY EMBANKMENT FROM EROSION DURING OVERTOPPING FLOOD EVENTS. SEE DITCH CHARTS ON THE PLAN SHEETS FOR THE DEPTH DIMENSION "D". CONSTRUCT ACCORDING TO TURF REINFORCED MAT SHOWN ON THE DETAIL SHEETS. CONSTRUCT ANCHOR TRENCHES AT POINTS 5, ENSURING OVERLAP LAP IS IN THE DIRECTION OF OVERTOPPING FLOW.
- 3 THE FOLLOWING ITEMS ARE INCIDENTAL TO PERFORATED PIPE EDGE DRAIN INSTALLATION (SEE KYTC SPECS-SECT. 704); TRENCH EXCAVATION, MATERIALS FOR BEDDING AND BACKFILL, AND GEOTEXTILE FABRIC FOR WRAPPING PERFORATED PIPE OR WRAPPING COARSE AGGREGATE BACKFILL

LITTLE STURGEON CREEK RD PAVEMENT DESIGN

1.50" CL2 ASPH SURF 0.38D PG64-22 3.00" CL2 ASPH BASE 1.00D PG64-22 3.00" CL2 ASPH BASE 1.00D PG64-22 4.00" CRUSHED STONE BASE 12.00" CRUSHED AGGREGATE SIZE NO 2

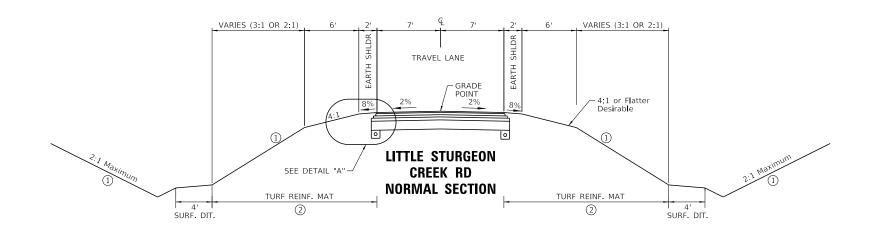
ENTRANCE PAVEMENT DESIGN

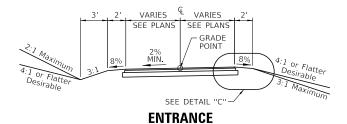
1.50" CL2 ASPH SURF 0.38D PG64-22

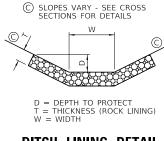
3.00" CL2 ASPH BASE 1.00D PG64-22 4.00" CRUSHED STONE BASE

EARTH 12 CRUSHED AGGREGATE SIZE NO 2 FABRIC-GEOTEXTILE CLASS 1 (OVERLAY AGGREGATE LAYER) NO. 57 STONE ③ FABRIC-GEOTEXTILE CLASS 2 ① 6" PERFORATED PIPE AT 0.30% MINIMUM SLOPE

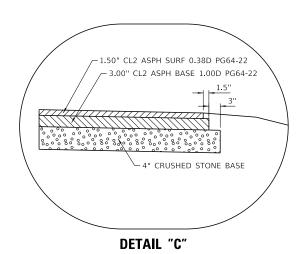
TYPICAL SECTIONS







DITCH LINING DETAIL



DETAIL "A"



DRAWING TITLE: LITTLE STURGEON CREEK TYPICAL SECTION

ITEM NO. COUNTY OF XX-XXXXXXX OWSLEY

OpenRoads Designer v23.00.01.11 USER: Min.llang

						6 6								
Corporate Limits			Main Water Marker	OWLM		Crash Cush i on TY 9		 >	Point (Misc)	_		Telephone Pedestal	TEL PED	
County Line	·		Main Water Greater Than 12	OWLMG12		Cross Notch	•NOTCH		Pole	•		Telephone Pole		-0-
Easement Fence COA		xx	Marker Sewer Sanitary			Curb Box In l et	CITTE		Pole (Light)	×		Temporary Benchmark		
Mineral Parcel		^^	Marker	OSSM		Curb Notch	●NOTCH		Post	•P0ST		Traffic Light	汞	早
Property Line			Sewer Sanitary Force Main Marker	OSANFMM		Combination Pole	Ī	古	Power Pole	<u> </u>		Traffic Signal Control Box	[]TSCB	
Right of Way Line			Sewer Storm			Delineator Post	•DP		Quarry	\bigotimes		Traffic Signal		
All Overhead			Marker	OSTMM		Drop Box	[]		Random (Ground Shot)	+		Junction Box	ĒĪŢSJВ	
Utility Lines Cable Underground	E (A) OE(A)		Multi Utility Bank Marker	OMUBM		Existing Spring		P	Railroad Mile Marker	•RRMM		Traffic Signal Pole	•	
Electric With Quality Levels	E (B)	——— Е ———	Oil Line Marker	OOLM		Electric Manhole	(EMH)	(EMH)	Railroad Spike	•RRS		Traverse Point	•TRAV	~
Duct Underground Electric With	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	EEE	Steam Line Marker	OSLM		Electric Meter	ОЕМ		R i ght of Way Marker			Tree	⇔	$\langle \cdot \rangle$
Quality Levels Cable Underground	= = $E(PA)$ = $=$ = $=$ $=$ $-$ =					Electric Pedestal	[]ELEC PED		Right of Way			TV Junction Box	_ JTV JB	
Fiber With Quality Levels	FO (B) FO (PA)	FO	Cable Guardrail			Electric Pole	•	\Box	Monument	(•	Utility Pole	•	-0-
Cable Underground	T (A) OT(A) T (B)		D i tch	——→···	$\longrightarrow \longrightarrow \longrightarrow$	Electric Junction Box	<u>□</u> EL JB		RR Traffic Signal Pole	•		Underground Storage Tank	(<u>05T</u>)	
Telephone With Quality Levels	T (CD)	<u>—т</u> —т	Edge of Water			Fire Hydrant	⊚		RW Parcel		P 000	Utility Test Hole		ΘTΗ
Duct Underground Telephone With	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	TTT	Fence Hedge	α		Flag Pole	•FP		Sanitary Cleanout	SSANCO		Water Line Marker	owlm.	
Quality Levels Cable Underground	= T (PA) = = = = = = = = = = = = =		Fence Flow Line/Thalweg/	X	X	Force Main	IXI		Sanitary Manhole	()SANMH	OSANMH	Water Meter	OWM	
TV With Quality Levels	TV (B)	TV	Int. Stream or Ditch			Sewer Valve Fuel Tank Inlet	⊙FT1		Satelite Dish	1SD		Water Spigot	ows	
Main Gas With	— — GM (A) — OGM(A) — — GM (B) — — — - — — GM (CD) — — -	GM	Guardrail		• • • • •	Fuel Tank Vent	OFTV		Septic Tank Cleanout	<i>ostc</i>		Water Valve	owv	oWV
Quality Levels Main Water	— — GM (PA) — — — -		Railroad			Gas Meter	ogM		Service Pole	•SP		Water We ll	oww	
With Quality Levels	WM (B)	⊢⊢⊢ WM ⊢	Shrub Line Sink Hole			Gas Monitoring			Sewer Air			Yard Light	¥	
Main Water	→ WM >12 (A) OWM>12(A) → WM >12 (B) → → ⊢		Tree Line			Well Gas Valve	≎GMW ≎GV	oGV	Release Valve	<u>*</u> I-←		Yard Sprinkler	@Y <i>S</i>	
reater Than 12 With Quality Levels	→ WM >12 (D) → → ⊢ → WM >12 (CD) ← → ⊢ → WM >12 (PA) ← → ⊢	WM >12	Wall (WSM or DSM)		\sim	Gas Vent	≎GVE	00V	Shrub			Yard Sprinkler	ev CIVIV	
Sewer Sanitary With Quality Levels	= = SAN (A) = OSAN(A) = = SAN (B) = = = = = = SAN (CD) = = = =	====SAN=====SAN=	Blue Line Stream	→···	→··· - *-	Gas Well	ogw		Sign	◆SIGN		Water Valve	⊚Y SWV	
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$		Lakes and Ponds			Guidewires &			Sign Post (Single)		Г			
Sewer Sanitary Force Main With Quality Levels	= = SAN FM (B) = = = = = = = = = = = = = = = = = = =	=====SAN FM=======	Regulated Floodway			Anchors		\dashv	Sign with 2 posts	00		<u>Utility</u>	<u>Owners</u>	
Sewer Storm With	= = STORM (A) OSTORM(A) = = STORM (B) = = = =	STORM	RDZ Line			Headstone	HEAD STONE		Sign group (4)	60d		Utilit JACKSON ENERO	y #1	D // TI\/ E
Quality Levels	=	STORM	ADA Ramp	&		Interstate Shield			Station Stamp	STATION STAMP		CORPORATIO	N - ELECT	RIC
Multi Utility Bank Quality Levels	= = MUB (A) = = OMUB(A) = = MUB (B) = = = = = = = = MUB (CD) = = = = = =	мив	Anchor Pole	•		Iron P i n	• <i>IP</i>		Storm Manhole	()SSMH		115 JACKSON McKee k	ENERGY L (Y 40447	LANE
·	= = MUB (PA) = = = = = = = = = = = = = = = = = = =		Benchmark	•		Light Pole	\times	¤	Stub Power	皇	효	CONTACT - JASC	N MONTG	OMERY
Oil Line Quality Levels	OIL (B)	STM	Bike Lane Symbol	0/10		Low Wire	+		Stub Te l ephone	•		606-87	'2-3903	
Steam Line	STM (A) OSTM(A) STM (B) STM (CD)	STM	Bollard	•B0LLARD		Mag Nail	•MAG		Survey Cross Notch	•CN			ty #2	
Quality Levels Cable Underground	STM (PA)		Centerline	+		Mailbox			Survey Curb Notch	•NOTCH		PEOPLES RUR COOPERATIVE		
Electric Marker	<i>○CUGEM</i>		Centerline Stationing	©		Manhole	€)MH	(EMH)	Survey Na il	●MAG		INC TE	ELEPHONE	
Duct Underground Electric Marker	ODUGEM		Control Monument	- -		Mile Marker Post	∘MP	M	Survey Spike	•RRS			OX 159 IN STREET	-
Cable Underground Fiber Marker	○CUGFM		Control Point	A		Mineral Parcel		M 000	Survey Stone Marker	•STONE		SOUTH McK CONTACT	ee KY 404	447
Cable Underground Telephone Marker	ocug⊤m		Core Hole	∞core		Misc Location Point		0	Swamp	<u> </u>			37-7101	INFL
Duct Underground	<i>○DUGTM</i>		Crash Cushion			Monitoring Well	OMW		Telephone Booth	ΞŒΒ				
Telephone Marker	ODUGI M		TY 6 D Crash Cushion			Parking Meter	∞ <i>PM</i>		Telephone Junction Box	[]TEL JB				
Cable Underground TV Marker	OCUGTVM		TY 6 A			Pedestrian Signal	C)PED SIG		Telephone Line					
Main Gas Marker	OGLM		Crash Cushion TY 9A			Pins/Pipes	• <i>IP</i>		Overhead					
						PK Na il	•PK		Telephone Manhole	CIMH3	TMH			

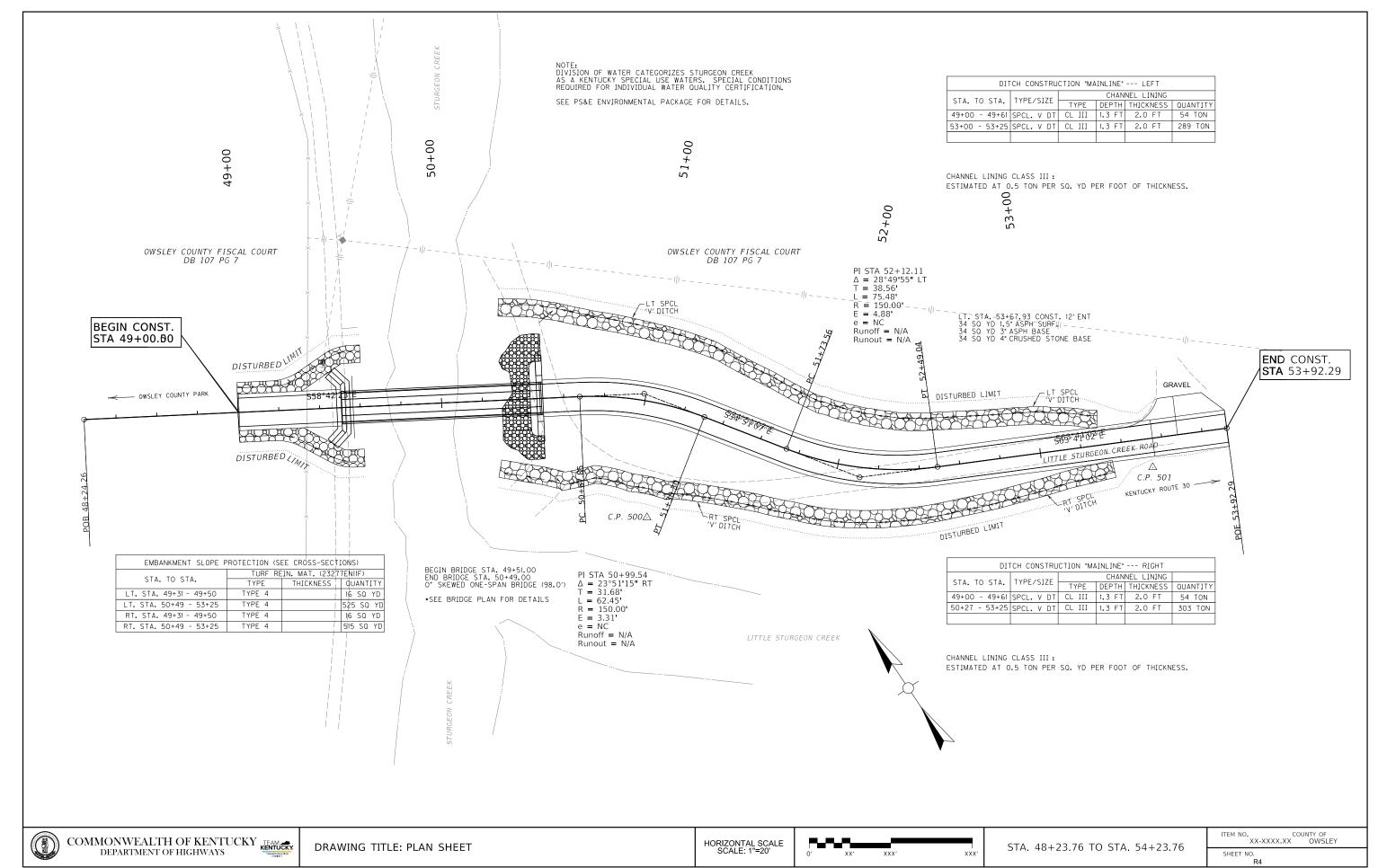
DRAWING TITLE: LEGEND & UTILITIES

ITEM NO. COUNTY OF XX-XXXXXXX OWSLEY

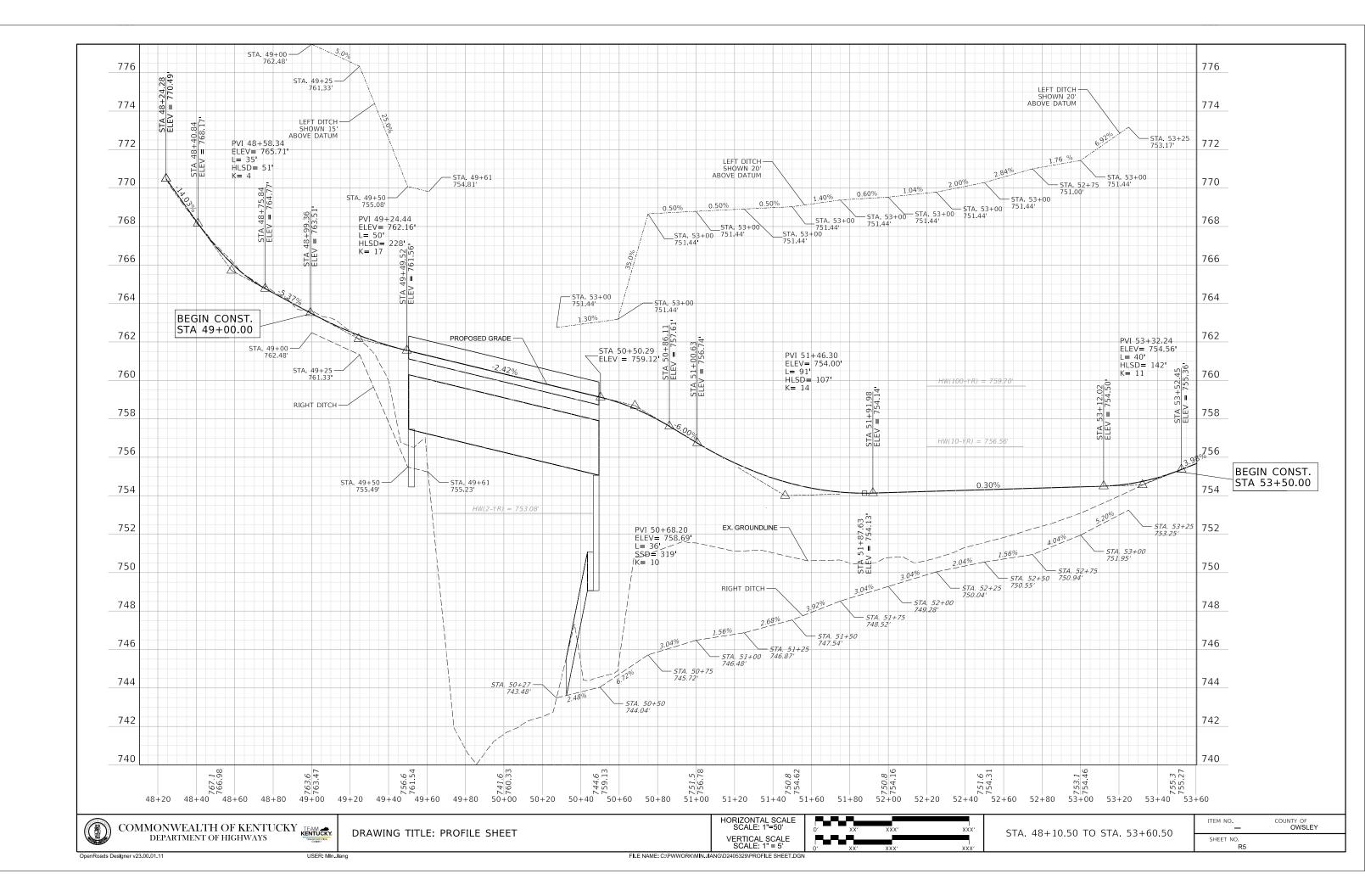
SHEET NO. R3

OpenRoads Designer v10.12.02.4

USER: Mln.Jlang



OpenRoads Designer v23.00.01.11 USER: Min.Jlang USER: Min.Jlang File NAME: C:IPWWORKiMIN.JlANG\D2405329\PLAN SHEET.DGN

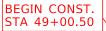


HORIZONTAL COORDINATE SYSTEM

Coordinates for Horizontal survey control were obtained from GPS-RTK methods utilizing the Kentucky CORS/VRS Network. Horizontal Coordinates are in U.S. Survey Feet.

Coordinates are based on the Kentucky State Plane Coordinate System, Single Zone (KY Zone 1600) and are in U.S. Survey Feet.





COORDINATE CONTROL POINTS

S58°42'23"E

POINT	DESCRIPTION	State Plane							
	DESCRIPTION	NORTH (Y) EAST (X)							
START	STA. 48 + 24.256	3703772.455 5483012.958							
PC	STA. 50 + 67.854	3703645.924 5483221.117							
PI	STA. 50 + 99.538	3703629.467 5483248.191							
PT	STA. 51 + 30.304	3703603.466 5483266.298							
PC	STA. 51 + 73.556	3703567.971 5483291.015							
PI	STA. 52 + 12.114	3705636.330 5483313.049							
PT	STA. 52 + 49.038	3703519.236 5483347.6 11							
END	STA. 53 + 92.294	3703455.727 5483476.020							

PI STA 50+ Δ = 23°51' Τ = 31.68'



E	= 3.3	1.
е	=	%
Ri	unoff =	
Ri	unout	_

COORDINATE CONTROL POINTS

	POINT	DESCRIPTION	State	Plane Coord	STATION 1 OFFS		
		DESCRIPTION	NORTH (Y)	EAST (X)	ELEV. (Z)	STATION and OFFSET	
	C.P. 500	I.P. W/KYTC CAP	3703578.245	5483215.340	750.408	51+16.87, 55.853RT	
	C.P. 501	I.P. W/ KYTC CAP	3703460.043	5483435.105	754.374	53 + 53.71, 14.270 RT	

PI STA 52+12.11 Δ = 28°49'55" LT T = 38.56' L = 75.48'

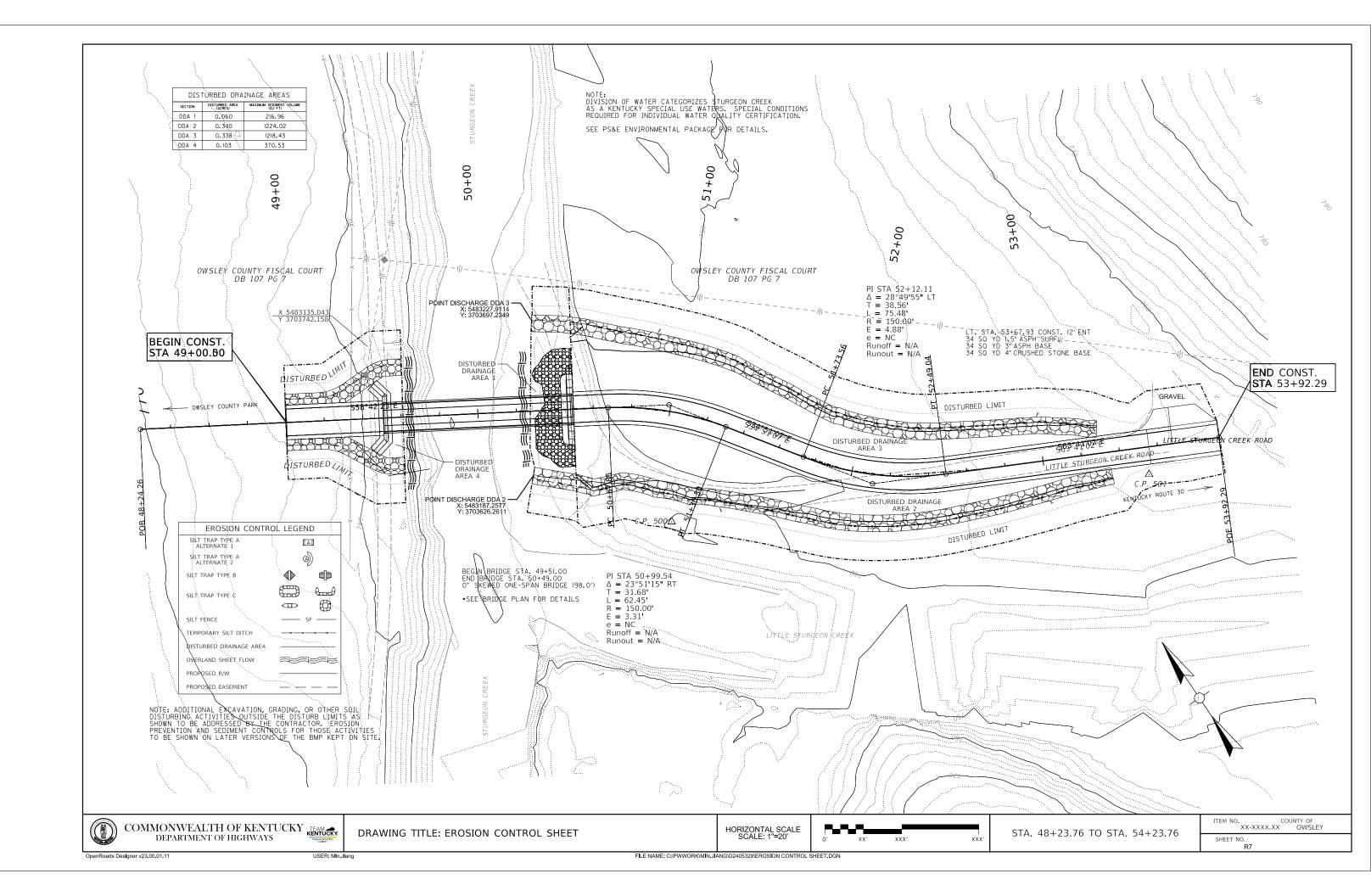
END CONST. STA 53+92.29

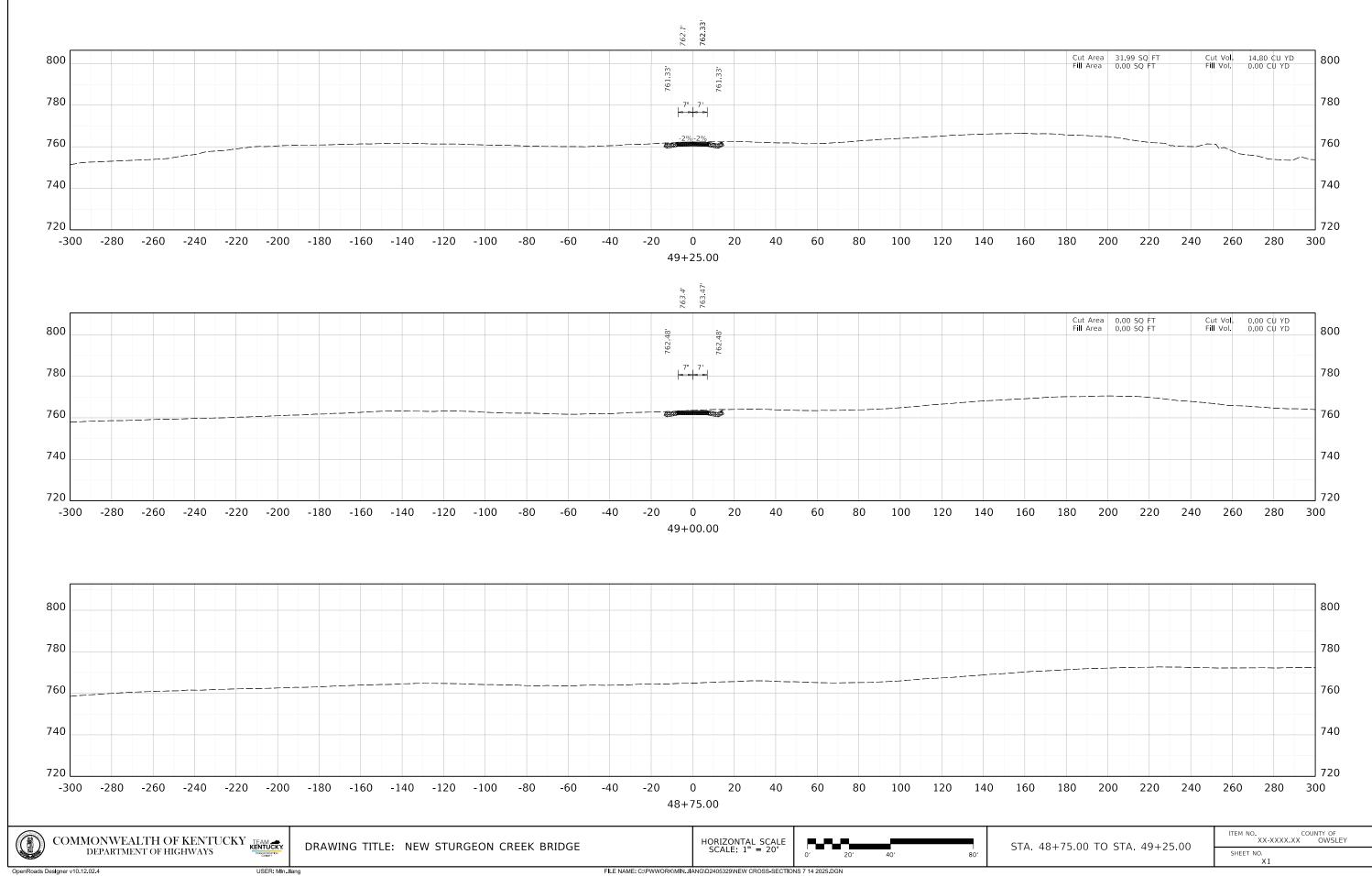
N CREEK ROAD

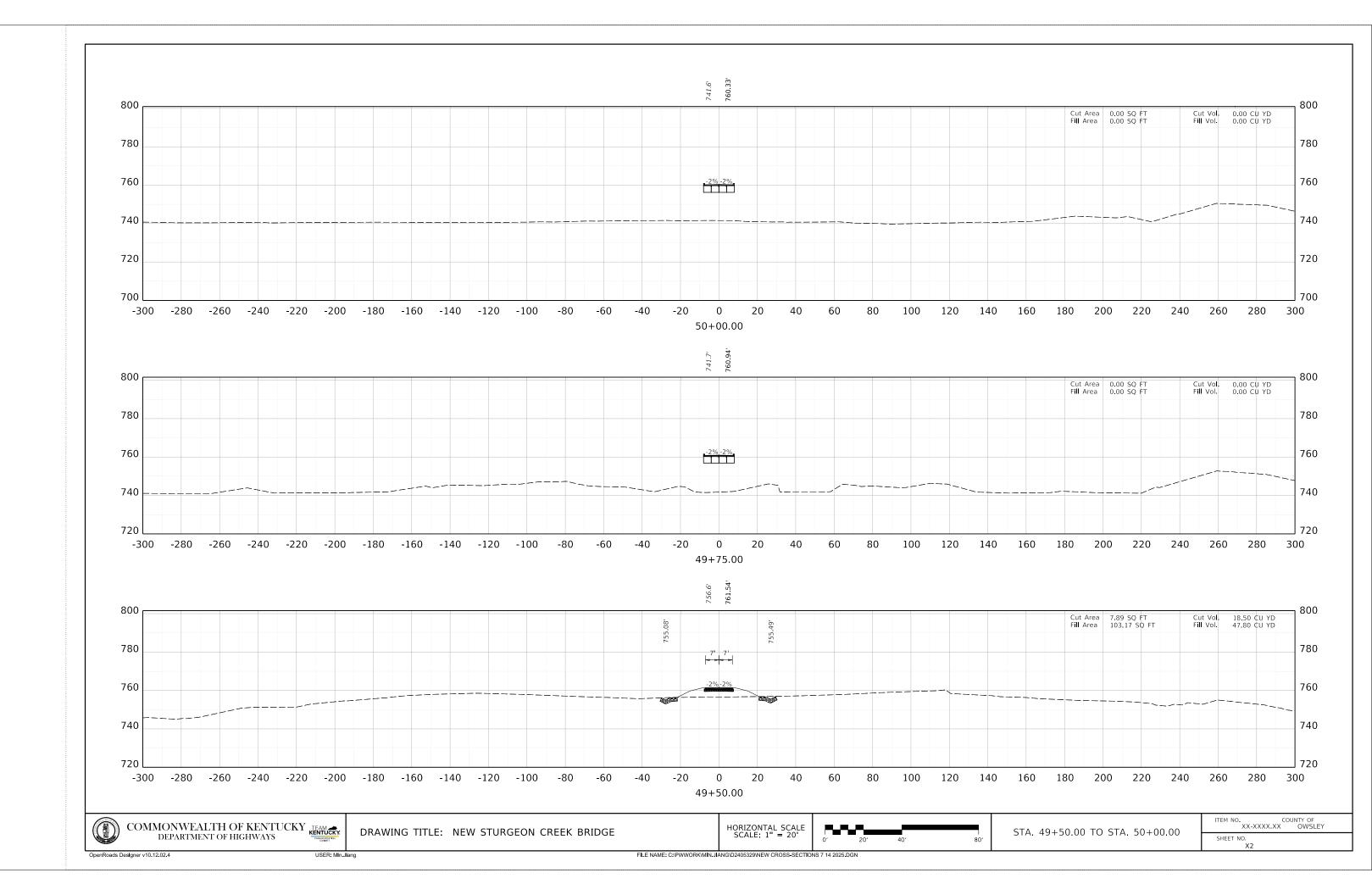
DRAWING TITLE: COORDINATE CONTROL SHEET

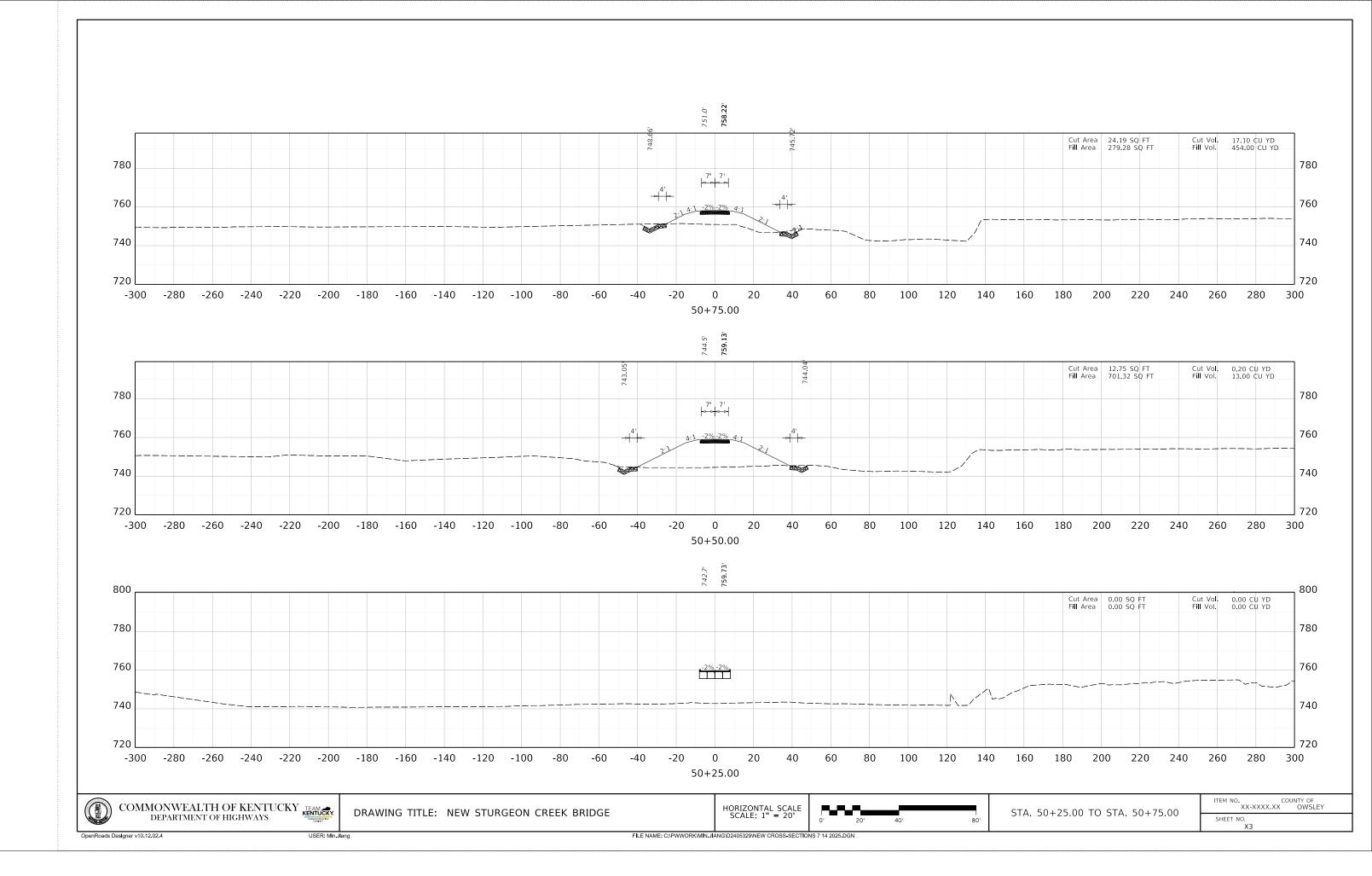
HORIZONTAL SCALE SCALE: 1"=20'

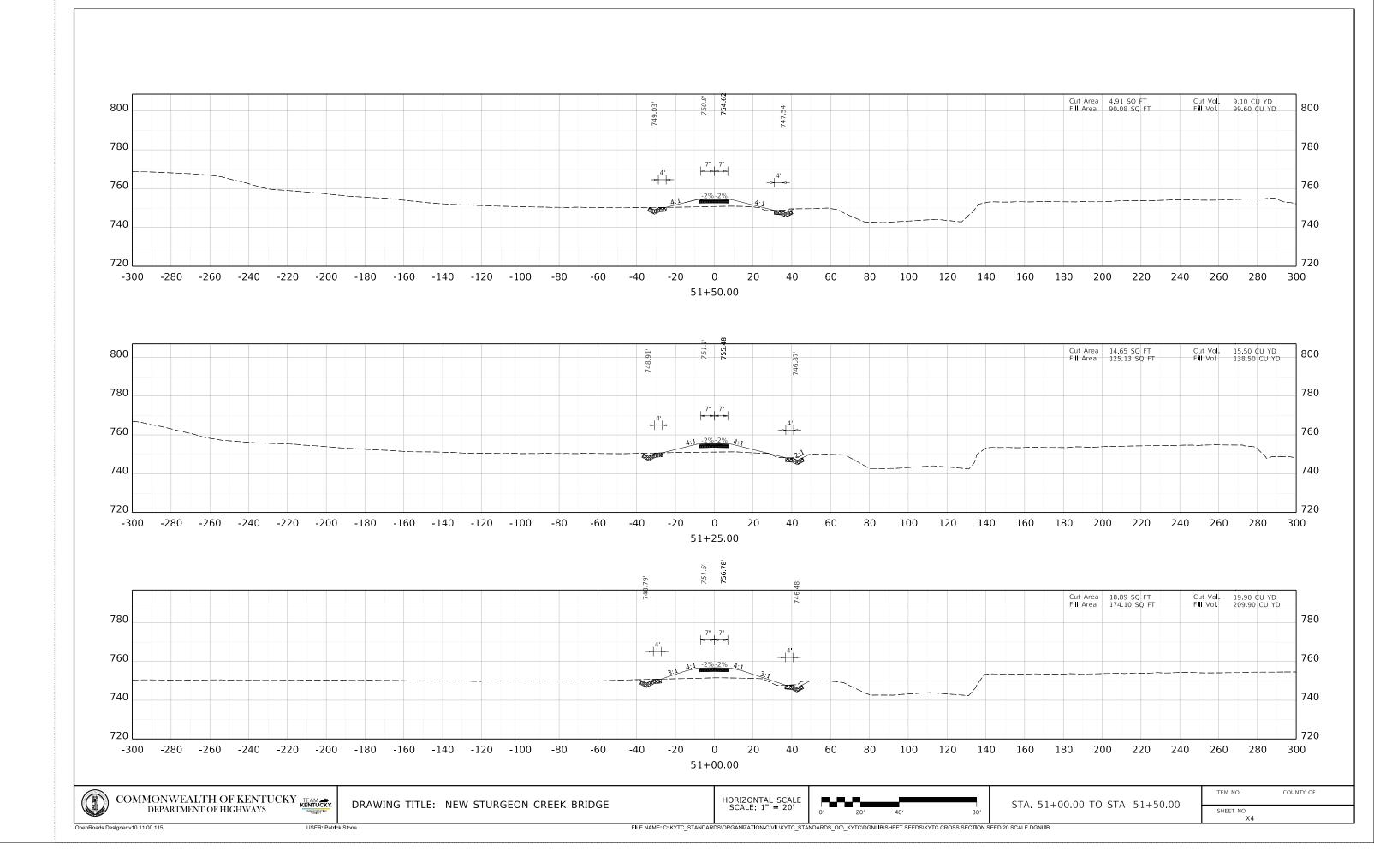
\$3_{4°51′07″E}

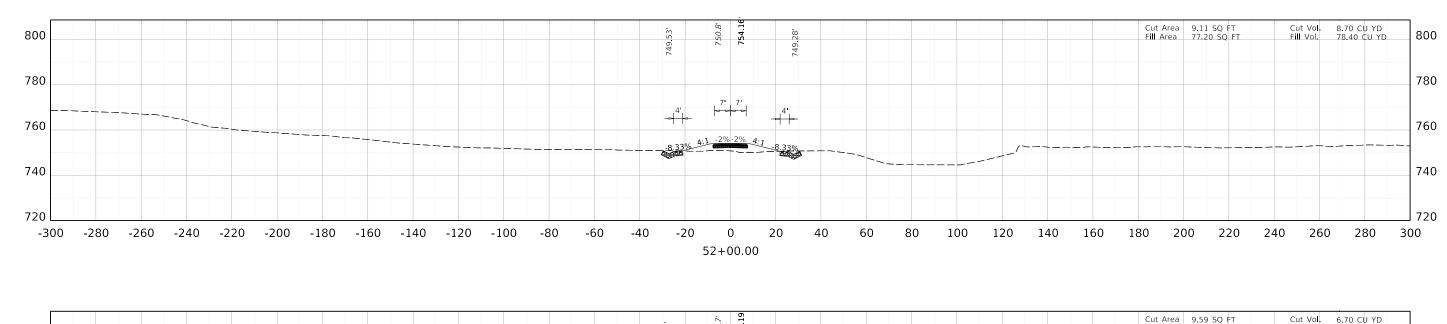


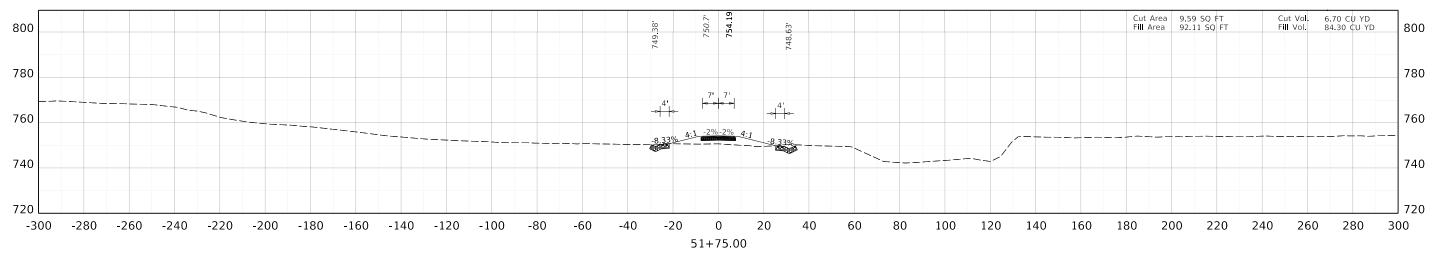












DRAWING TITLE: NEW STURGEON CREEK BRIDGE

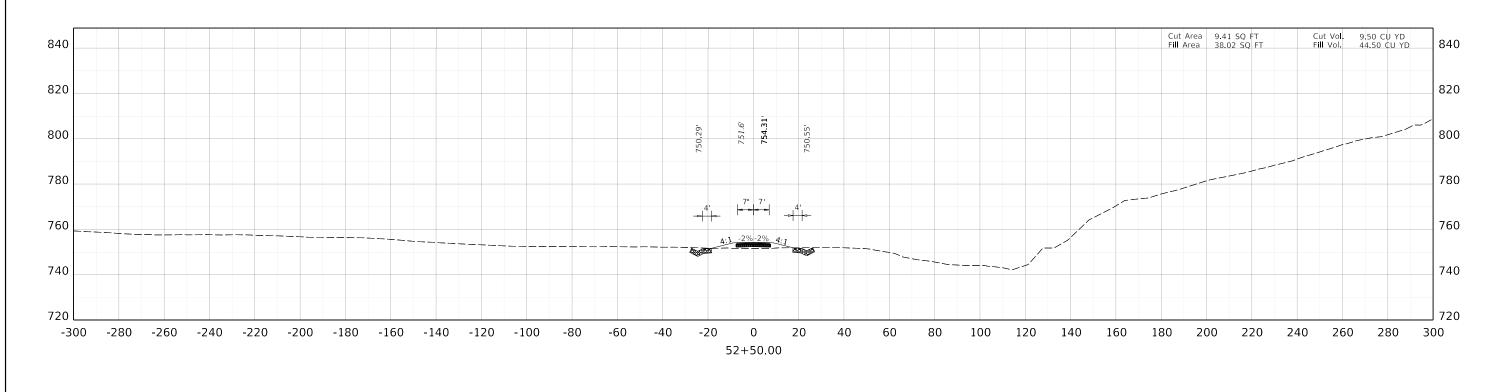
HORIZONTAL SCALE SCALE: 1" = 20' 20' 40' 80'

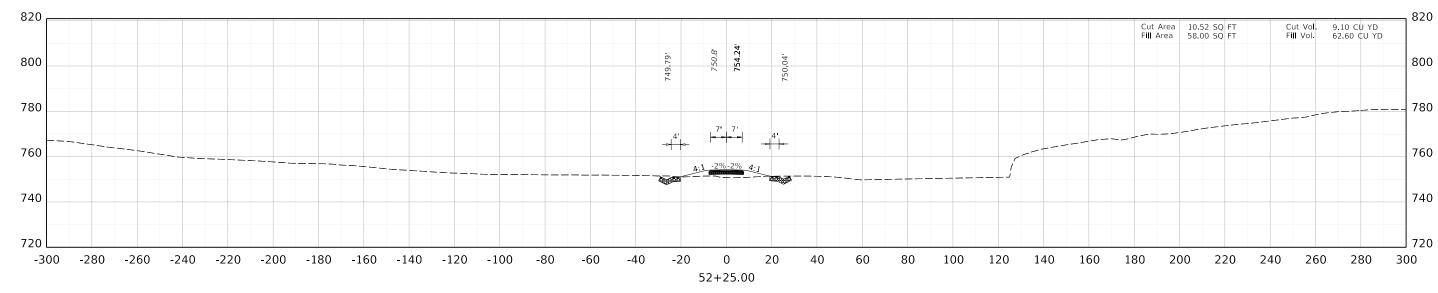
STA. 51+75.00 TO STA. 52+00.00

TEM NO. COUNTY OF

SHEET NO.

X5



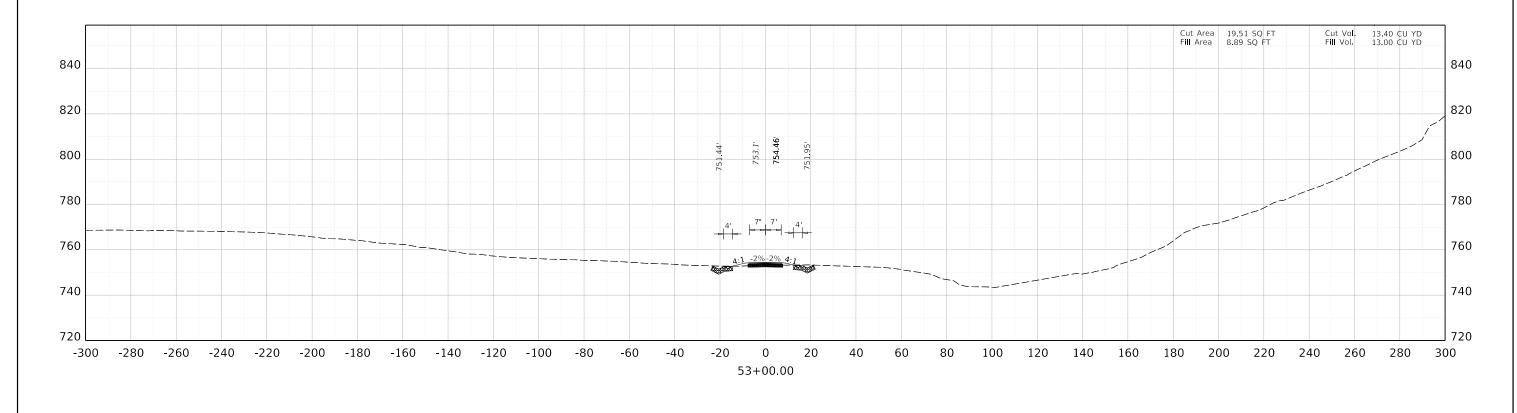


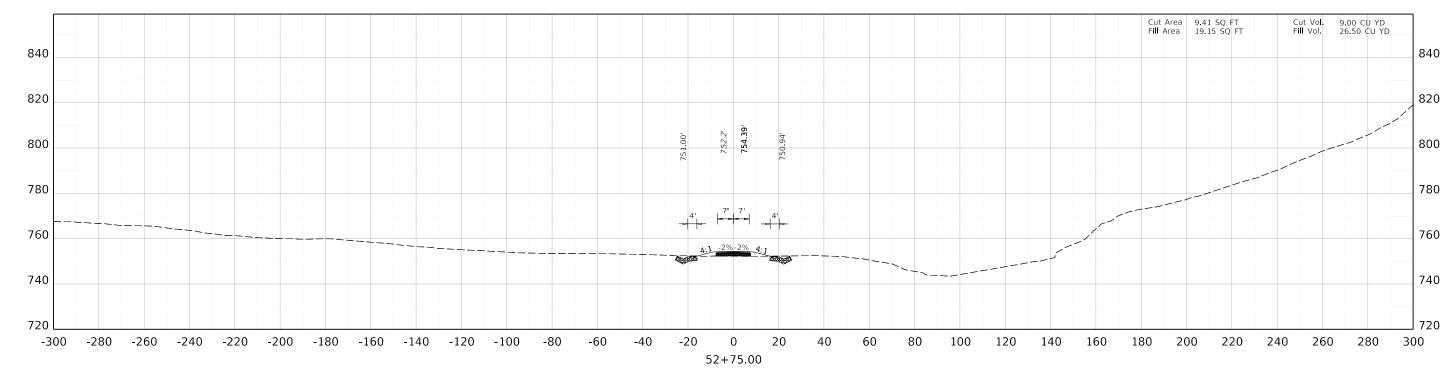
DRAWING TITLE: NEW STURGEON CREEK BRIDGE

HORIZONTAL SCALE SCALE: 1" = 20' 20' 40' 80'

STA. 52+25.00 TO STA. 52+50.00

SHEET NO. X6





DRAWING TITLE: NEW STURGEON CREEK BRIDGE

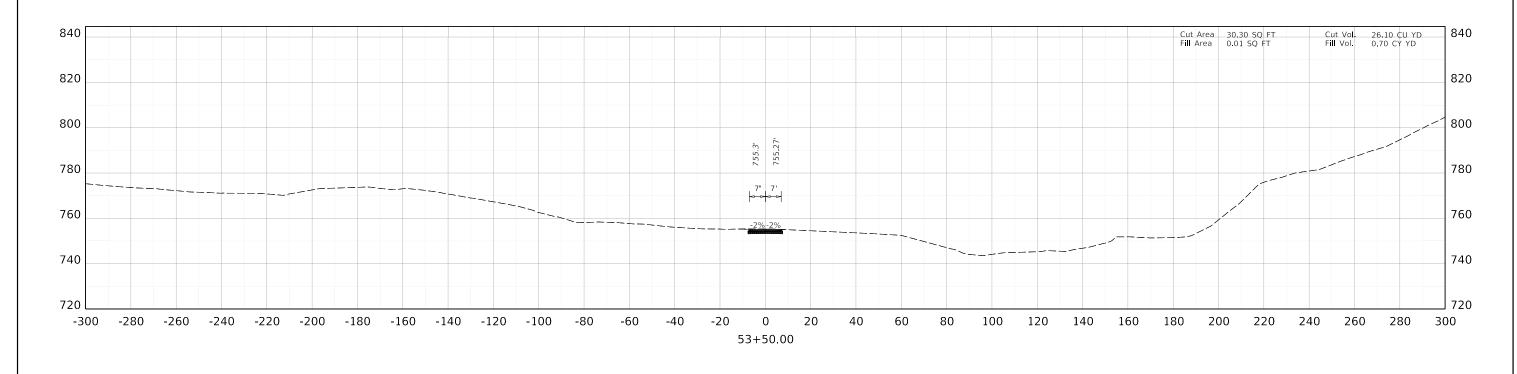
HORIZONTAL SCALE SCALE: 1" = 20' 20' 40' 80'

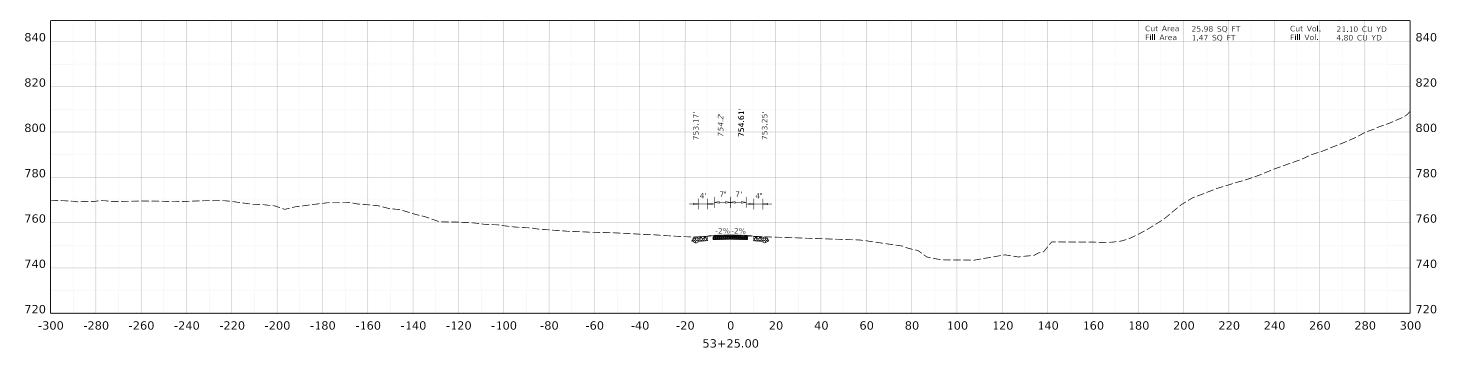
STA. 52+75.00 TO STA. 53+00.00

ITEM NO. COUNTY OF

SHEET NO.

X7





DRAWING TITLE: NEW STURGEON CREEK BRIDGE

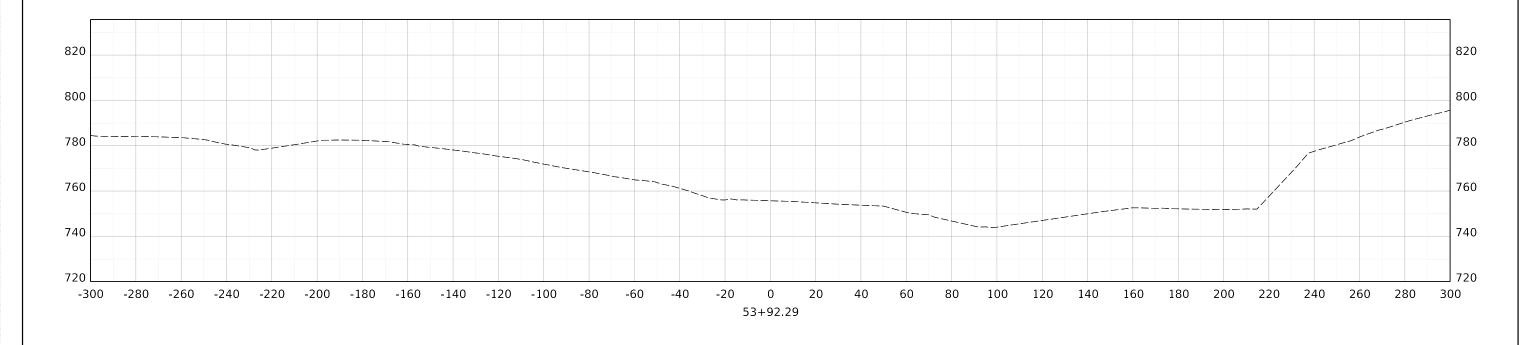
HORIZONTAL SCALE SCALE: 1" = 20' 20' 40' 80'

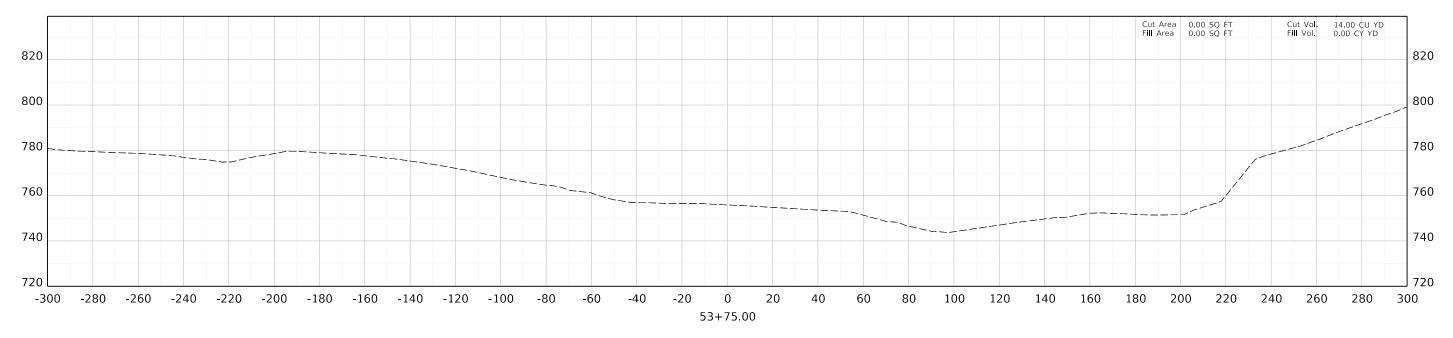
STA. 53+25.00 TO STA. 53+50.00

TEM NO. COUNTY OF

SHEET NO.

X8





DRAWING TITLE: NEW STURGEON CREEK BRIDGE

HORIZONTAL SCALE SCALE: 1" = 20' 20' 40' 80'

STA. 53+75.00 TO STA. 53+92.29

ITEM NO. COUNTY OF XX-XXXX.XX OWSLEY

SHEET NO. X9

TRANSPORTATION CABINET DEPARTMENT OF HIGHWAYS

OWSLEY COUNTY STURGEON CREEK RD. OVER STURGEON CREEK STA. 50+00.00

	ESTIMATE OF QUANTITIES																				
BID ITEM CODE	08100	08104	08150	08151	08002	02231	23378EC	08666	08046	08033	08003	03299	08019	26233EC	08094						
BID ITEM	Concrete Class "A"	Concrete Class "AA"	Steel Reinforcement	Steel Reinforcement, Epoxy Coated	Structure Excavation, Solid Rock	Structure Granular Backfill	Concrete Sealing	PPC Box Beam CB42	Piles - Steel HP 12 x 53	Test Piles	Foundation Prep	Armored Edge for Concrete	Cyclopean Stone Rip Rap	Mobilization for Concrete Surface Treatment	Pile Points 12 Inch						
UNIT	C.Y.	C.Y.	LBS.	LBS.	C.Y.	C.Y.	S.F.	L.F.	L.F.	L.F.	L.S.	L.F.	TON	L.S.	EA.						
Abutment #1	32.3		2801		38		315														
End Bent #2	28.3	8.6		2647		135	381		118	28			169		6						
[e]																					
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Substructu	ļ																				
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Supplementary and the second	 	28.3		3489			1846	202				32									
Superstructure	_	28.3		3489			1846	392				32									
BRIDGE TOTALS	60.6	36.9	2801	6136	38	135	2542	392	118	28	1	32	169	1	6						

		IND	EX OF SHE	ETS
Sheet N	No.		Descripti	
S1		itle Sheet	·	
S2	2 G	eneral Not	tes	
S3		ayout		
S4		ubsurface oundation		
S5		oundation butment #		
S7		itegral End	•	
S8			General Notes	
S9			Box Beam Bearing	Details
S10		uperstruct		
S1	1 C	onstruction	n Elevations	
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BGX-0	S	STAND	DARD DRA	tures WINGS
BGX-0	S 12-02 03-02	STAND Geotechnic Elastomeric	DARD DRA al Legend Bearing Pads for Box E	tures WINGS
BGX-0 BBP-00 BGX-0	S 12-02 03-02 06-10	STAND Geotechnic Elastomeric Stencils for	OARD DRA al Legend Bearing Pads for Box E	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00	S 12-02 03-02 06-10 01-14	STAND Geotechnic Elastomeric Stencils for Neoprene E	DARD DRA' lal Legend : Bearing Pads for Box E Structures xpansion Dams and Arr	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0	S 12-02 03-02 06-10 01-14 03-03	Geotechnic Elastomeric Stencils for Neoprene E Box Beam N	DARD DRA' lal Legend : Bearing Pads for Box E Structures xpansion Dams and Arr Misc, Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00	S 12-02 03-02 06-10 01-14 03-03 04-04	STAND Geotechnic Elastomeric Stencils for Neoprene E Box Beam N Box Tensior	DARD DRA' lal Legend : Bearing Pads for Box E Structures xpansion Dams and Arr	WINGS Beams
BGX-0 BBP-0 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	STAND Geotechnic Elastomeric Stencils for Neoprene E Box Beam N Box Tensior	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GENERAL BRIGARIAN CONTROL OF THE PROPERTY OF T	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm Misc, Details Rod Details CB42 Details	WINGS Beams
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BGX-0 BBP-00 BGX-0 BJE-00 BDP-0 BDP-0	S 12-02 03-02 06-10 01-14 03-03 04-04 12-04	GEOTECHNIC Geotechnic Elastomeric Stencils for Neoprene E Box Beam I Box Tension HP 12x53 S	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arr Misc. Details CB42 Details teel Pile	WINGS Beams mored Edges
BGX-0 BBP-00 BJE-00 BDP-0 BDP-0 BDP-0	SS 12-02 03-02 06-10 101-14 03-03 04-04 12-04 03-09	GTAND Geotechnic Elastomeric Stencils for Neoprene E Box Beam 1 Box Tenslor Box Beam 6 HP 12x53 S	DARD DRAY Cal Legend Bearing Pads for Box E Structures Expansion Dams and Arm Misc, Details Rod Details CB42 Details teel Pile	WINGS Beams mored Edges ONS
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BGX-0 BBP-00 BJE-00 BDP-0 BDP-0 BDP-0	SS 12-02 03-02 06-10 101-14 03-03 04-04 12-04 03-09	GTAND Geotechnic Elastomeric Stencils for Neoprene E Box Beam N Box Tension Box Beam O HP 12x53 S	DARD DRAY Cal Legend Bearing Pads for Box E Structures Expansion Dams and Arm Misc, Details Rod Details CB42 Details teel Pile	WINGS Beams mored Edges ONS
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BGX-0 BBP-00 BJE-000 BDP-0 BDP-0 BPS-00	Standa Constru AASHT	GTAND Geotechnic Elastomeric Stencils for Neoprene E Box Beam (HP 12x53 S) SP and Specific uction.	DARD DRA al Legend Bearing Pads for Box E Structures xpansion Dams and Arm whisc, Details Rod Details Leel Pile ECIFICATI ations for Road and	WINGS Beams mored Edges ONS I Bridge cations COUNTY OF
BGX-0 BBP-00 BJE-000 BDP-0 BDP-0 BPS-00	Standa Constru AASHT	GTAND Geotechnic Elastomeric Stencils for Neoprene E Box Beam (HP 12x53 S) SP ord Specific uction. O LRFD Bri	DARD DRAY al Legend Bearing Pads for Box E Structures xpansion Dams and Arr Misc. Details Rod Details CB42 Details teel Pile ECIFICATI ations for Road and	WINGS Beams mored Edges ONS Bridge Cations

GENERAL NOTES

CONCRETE SEALER: All areas detailed in the specifications as requiring masonry coating shall be sealed in accordance with the special note for concrete sealing. The superstructure deck, barriers and overhangs shall also be sealed as shown herein these plans. Concrete surfaces (except the deck) shall receive the ordinary surface finish as described in section 601.03.18(A) prior to being sealed.

SPECIFICATIONS: All references to the Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction with current Supplemental Specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specs, with interims.

DESIGN LOAD: This bridge is designed for a KYHL-93 live load. The KYHL-93 live load is arrived at by increasing the standard HL-93 truck and lane loads as specified in the AASHTO Specifications by 25%.

FUTURE WEARING SURFACE: This structure is designed for a 15 PSF future wearing surface load.

DESIGN STRESSES: Concrete Class "A" f'c = 3500 psiConcrete Class "AA" f'c = 4000 psi Steel Reinforcement Fv = 60,000 psi

Structural Steel Yield Strength Fv = 50.000 psi

DESIGN METHOD: All reinforced concrete members are designed by the load and resistance factor method as specified in the current AASHTO Specifications.

WIND LOAD: This bridge is designed for a wind load based on a wind velocity of 100 mph.

REINFORCEMENT: Dimensions shown from the face of concrete to bars are to center of bars unless otherwise shown. Spacing of bars is from center to center of bars. Clear distance to face of concrete is 2", unless otherwise noted. Any reinforcement bars designed be suffix (e) in the plans shall be epoxy coated in accordance with section 811.10 of the Standard Specifications. Any reinforcing bars designated by suffix (s) in a bill of reinforcement shall be considered a stirrup for purposes of bend diameters

BEVELED EDGES: Bevel all exposed edges ³/₄" unless otherwise noted.

COMPLETION OF THE STRUCTURE: The Contractor is required to complete the structure in accordance with the plans and specifications. Material, labor or construction operations, not otherwise specified, are to be included in the bid item most appropriate to the work involved. This may include cofferdams, shoring, excavations, backfilling, removal of all or parts of existing structures, phase construction, incidental materials, labor or anything else required to complete the structure.

SHOP DRAWINGS: Submit shop drawings that are required by the plans and specifications directly to the Division of Structural Design. Is any changes in the design plans are proposed by a fabricator or supplier, submit those changes to the Department through the Contractor.

FOUNDATION DATA: See Foundation Layout Sheet.

DIMENSIONS: Dimensions are for a normal temperature of 60 degrees Fahrenheit. Layout dimensions are horizontal dimensions.

SUPERSTRUCTURE SLAB: Ensure the entire superstructure slab is poured continuously, out to out, before allowing any concrete to set.

PILE POINTS: Provide pile points for all point bearing piles. Ensure pile points are in accordance with Section 604 of the Specifications and of the type as shown on the Foundation Layout Sheet,

SLOPE PROTECTION: Use dry cyclopean stone slope protection in accordance with the plans and Specifications. Geotextile Fabric is to be incidental to this item.

MASONRY COATING: Contrary to the Specifications, do not apply Masonry Coating. Apply Concrete Sealing in place of Masonry Coating as noted in CONCRETE SEALER note.

The following abbreviations may have been used in the preparation of these plans:

between

Back Face

BOF Bottom of Footing BOS Bottom of Slab bot. **Bottom** Brg. Bearing Center to Center C to C Current Edition c.e. C.Y. Cubic Yards Chd. Chord CL Center Line Clr. Clear Concrete Conc. Cu. Cubic Dwa Drawing e.f. Each Face EI. Elevation eq. Equal Est. Estimate Ext. Exterior F to F Face to Face f.f. Front Face f.s. Far Side Feet I.D. Inside Diameter Inch Int. Interior

bet.

b.f.

LBS

n.s.

Low Bridge Seat LBS. Pounds Meter MPH Miles Per Hour

O.D. Outside Diameter Opp. Opposite Point of Curvature Perp. Perpendicular

Point of Intersection ы Precast Prestressed Concrete PPC

PPCDU Precast Prestressed Deck Unit PSI Pounds per Square Inch PT Point of Tangency

Near Side

Radius

Riaht

RCBC Reinforced Concrete Box Culvert RCDG Reinforced Concrete Deck Girder

ROUTE

Sturgeor Creek Rd OWSLEY

28995

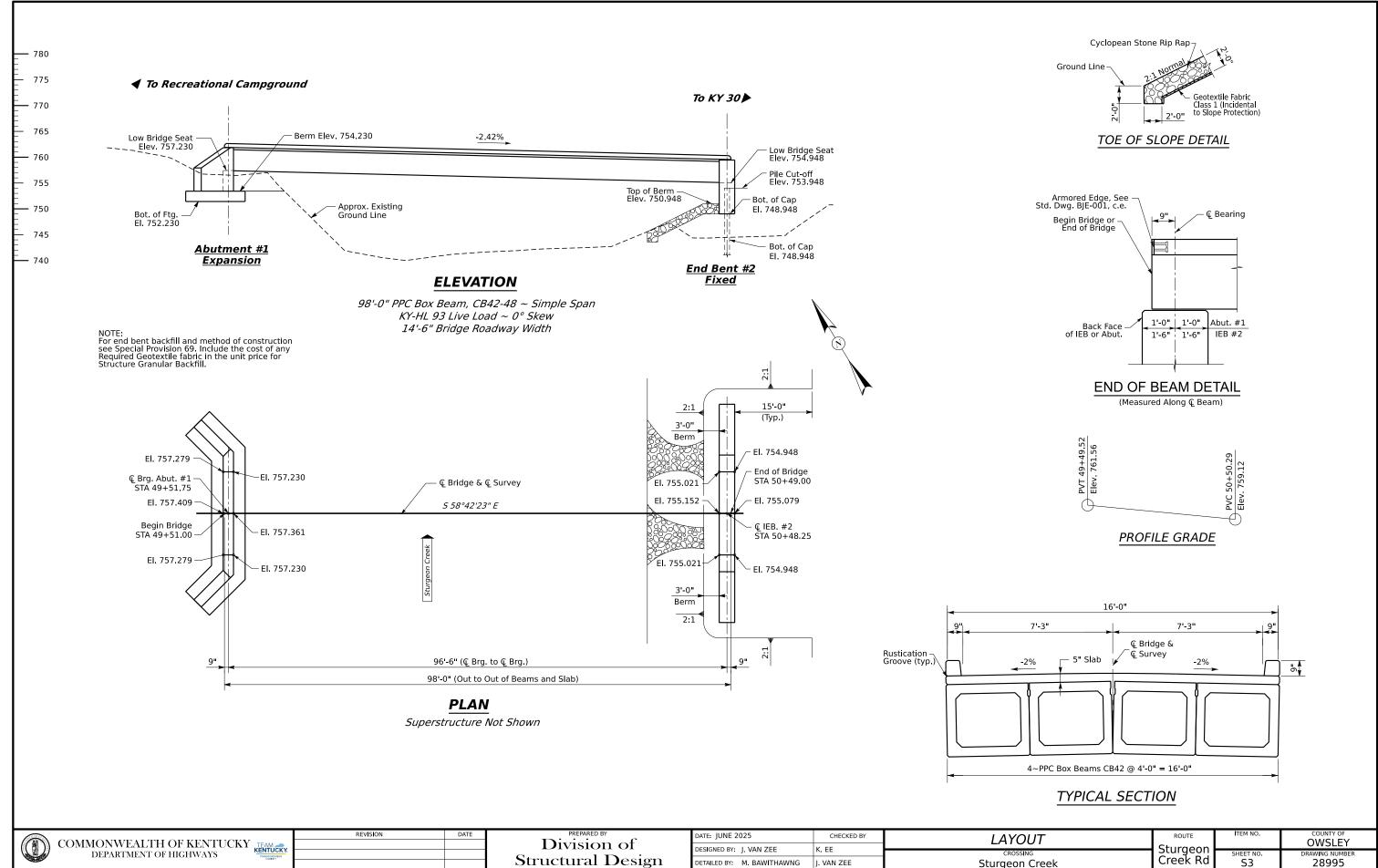
Rea'd Required RR Railroad Shld. Shoulder Spaces spa. Sta. Station Std Standard Str Straight Tan Tangent Thru Through TOF Top of Footing TOS Top of Slab Tot. Total Тур. Typical

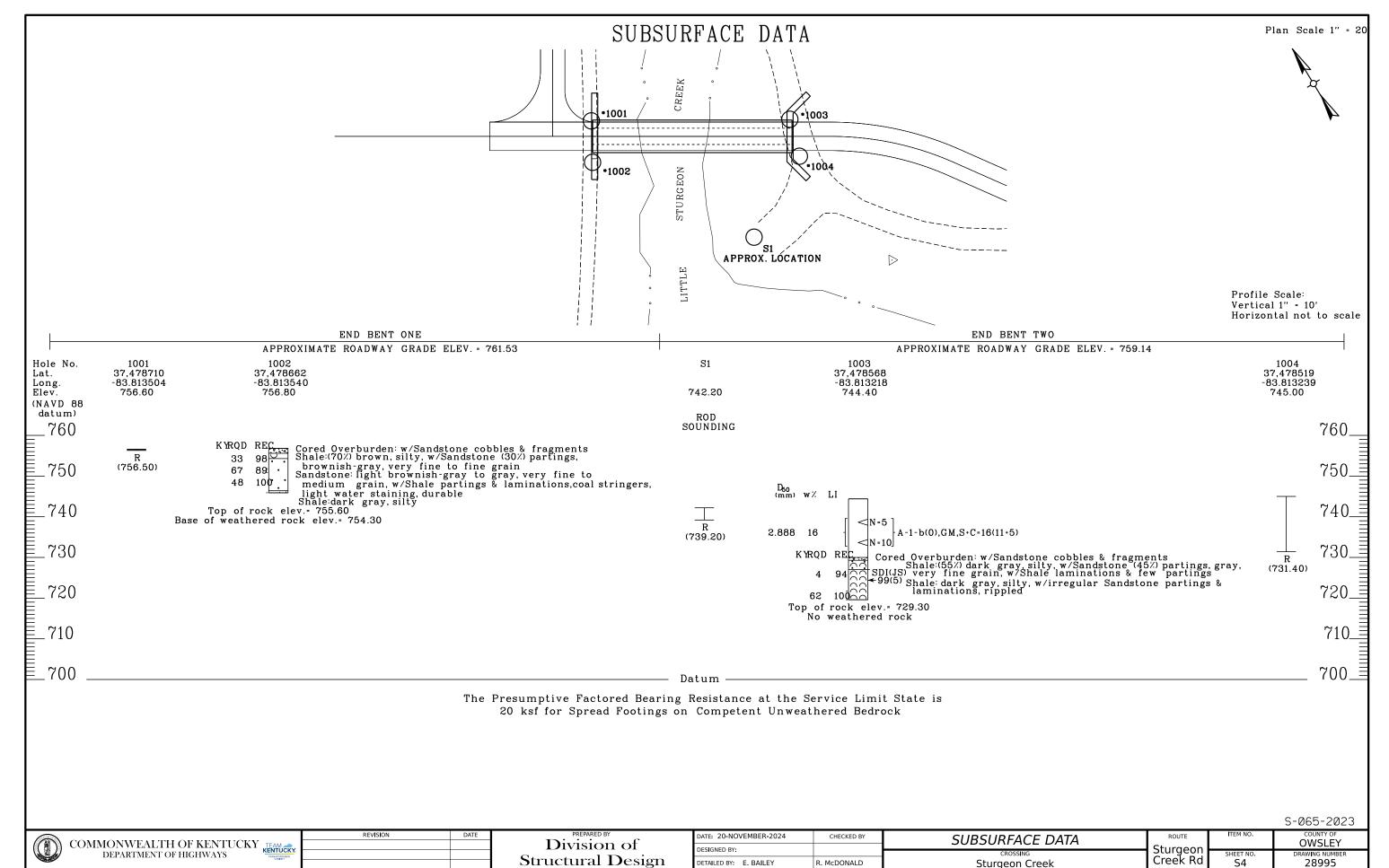
Vert. Vertical W.P. Working Point Yard

GENERAL NOTES

Sturgeon Creek

	REVISION	DATE	PREPARED BY	DATE: JUNE 2025	CHECKED BY
COMMONWEALTH OF KENTUCKY			Division of	DESIGNED BY: J. VAN ZEE	K. EE
DEPARTMENT OF HIGHWAYS			Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE
MicroStation v10.16.3.31 USER: mes	siah bawithawn DATE PLOTTED	16-JUL-2025	FILE NAME: J:\District10\RSnI	M\Owsley Sturgeon Creek\Details quantities	closeout\28995.dgn





Spread Footing Record Abutment #1

Point	Plan Footing Elevation	As-Built Footing Elevation
Α	752.230'	
В	752.230	
C.	752.230'	

Footing is designed for a maximum pressure of 8 KSF.

The allowable bearing capacity is 20 KSF.

The Project Resident Engineer is to record the "As-Built Footing Elevation" taken at the bottom of footing and submit one copy of this sheet to:

Kentucky Transportation Cabinet Division of Structural Design Room #322 200 Mero Street Frankfort, KY 40622

If the spread footing foundation is stepped due to unsuitable material found at the given elevation, record the location and elevation of the step as well.

ABUTMENT #1 FOOTING

All footing excavations in bedrock shall be cut neatly so that no forming or backfilling is necessary in the construction of the portions of the footings located in rock. Concrete should be placed directly against the cut rock faces. Mass concrete should be placed in the excavation from the top of the footing to the bedrock surface where the footing does not extend to the bedrock surface.

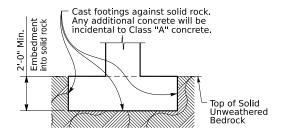
The bedrock at this location is susceptible to weathering and softening in the presence of water. Water must be kept out of the footing excavations. The footing steel and concrete should be placed the same day as or as soon as practical after the footing excavation is made. If the bedrock becomes softened at bearing elevation, the softened material should be undercut to unweathered material prior to placing the concrete.

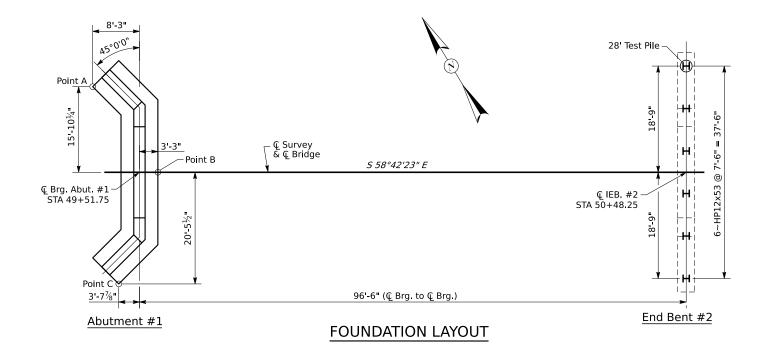
If the bedrock becomes softened at bearing elevation, the softened material should be undercut to unweathered material prior to placing the concrete. Seasonal groundwater fluctuations may cause groundwater infiltration into the footing excavations and a dewatering method may be necessary.

The spread footings shall be embedded a minimum of 2.0 feet into competent unweathered bedrock. Footings may be raised if competent unweathered bedrock is encountered at a higher elevation. (Note: Minimum 2.0 feet of embedment must still be attained.)

Solid rock excavation will be required for installation of this structure's spread footings.

Sheeting, Shoring, Cofferdams and/or dewatering methods will be required to facilitate foundation construction of spread footings. All costs incidental to lump sum bid for Foundation Preparation.





DRIVING CRITERIA

PRACTICAL REFUSAL: Drive point bearing piles to practical refusal. For this project minimum blow requirements are reached after total penetration becomes $\frac{1}{2}$ inch or less for 10 consecutive blows, practical refusal is obtained after the pile is struck an additional 10 blows with total penetration of $\frac{1}{2}$ inch or less. Advance the production piling to the driving resistances specified above and to depths determined by test piles) and subsurface data sheets). Immediately cease driving operations if the pile visibly yields or becomes damaged during driving. If hard driving is encountered because of dense strata or an obstruction, such as a boulder before the pile is advanced to the depth anticipated, the Engineer will determine if more blows than the average driving resistance specified for practical refusal is required to further advance the pile. Drive additional production and test piles if directed by the Engineer.

HAMMER CRITERIA: A hammer with a rated energy of between 25 and 35 kip-ft will be required to drive the H-piles to practical refusal without encountering excessive blow counts or damaging the pile. The contractor shall submit the proposed pile driving system to the Department for approval prior to the installation of the first pile. Approval of the pile driving system by the Engineer will be subject to satisfactory field performance of the pile driving procedures.

PII	PILE RECORD FOR POINT BEARING PILES											
Pile No.	Elevation	Pile Length In Place	Point of Pile Elevation As Driven	Design Axial Load								
	FEET	FEET	FEET	TONS								
	End Bent #2											
1	753.948			93								
2	753.948			93								
3	753.948			93								
4	753.948			93								
5	753.948			93								
6	753.948			93								
7	753.948			93								

FIELD DATA

For each pile, the Project Engineer shall record the following on this sheet: Pile Length in Place and Point of Pile Elevations as Driven.

Submit this record to:

Kentucky Transportation Cabinet Division of Structural Design 3rd. Floor East 200 Mero Street Frankfort, KY 40622

This pile record does not replace other pile records the Project Engineer is required to keep and submit.

DEFINITIONS OF TERMS

PILE CUT-OFF ELEVATION: Elevation of the top of pile in the finished structure.

PILE LENGTH IN PLACE: Actual pile length below the Pile Cut-Off Elevation in the finished structure.

PILE TIP ELEVATION AS DRIVEN: Actual point of pile elevation in the finished structure.

DESIGN AXIAL LOAD: Load carried by each pile as estimated from structural design calculations for Factored LRFD Loadings.

CALCULATED FIELD BEARING: Contrary to Section 604.03.07 of the Standard Specifications, in place bearing values are not required for piles bearing on rock when driven to practical refusal.

GENERAL NOTES

SPECIFICATIONS: Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, current edition.

PILE Points: Provides Pile Points capable of breaking boulders and seating into sloping rock surface for all piles.

	COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS	TEAM
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REVISION DATE

Division of Structural Design
 DATE: JUNE 2025
 CHECKED BY

 DESIGNED BY: J. VAN ZEE
 K. EE

 DETAILED BY: M. BAWITHAWNG
 J. VAN ZEE

FOUNDATION LAYOUT

CROSSING
Sturgeon Creek

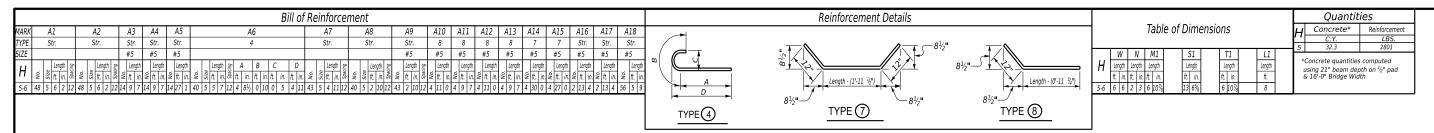
Sturgeon Creek Rd S5

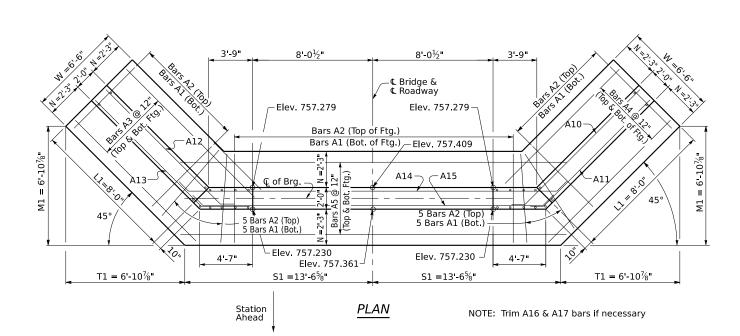
OWSLEY

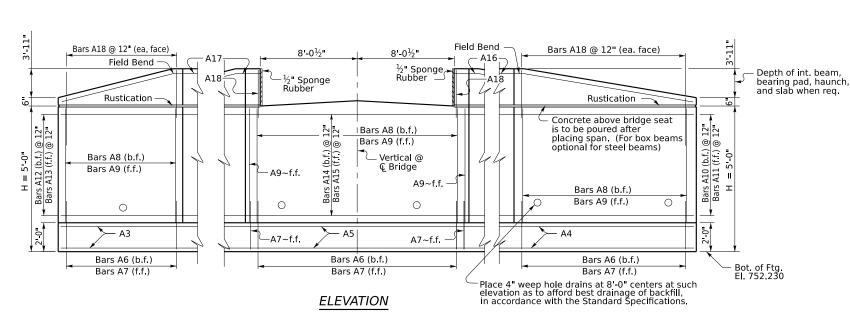
28995

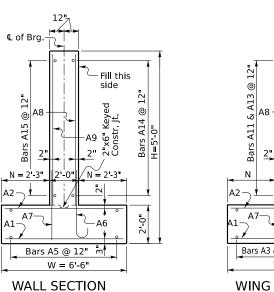
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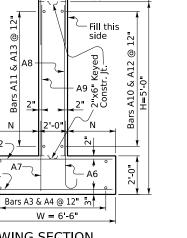
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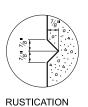








-A16 or A17



GROOVE

WING SECTION

GENERAL NOTES

SPECIFICATIONS: Construct abutments according to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction.

Abutments are designed for side by side box beams as detailed in Standard Drawings BDP-001 through BDP-012, current edition. Dimensions may be adjusted to allow for 17 6" rolled steel beam bridge width.

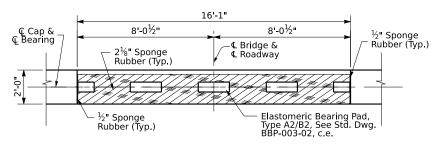
FOUNDATION PRESSURE: Construct abutment footings on solid rock bearing material that can support a pressure of 8000 psf service or 10,800 psf strength factored as recommended by a geotechnical engineer.

WING LENGTHS: Calculated assuming 21° superstructure depth and stream bank

FOOTING ELEVATION: Construct bottom of footing below the anticipated scour elevation. (This typically entails embedding the footings 1'-0" to 2'-0" into rock and pouring concrete directly against cut rock faces as recommended by geotechnical engineer.)

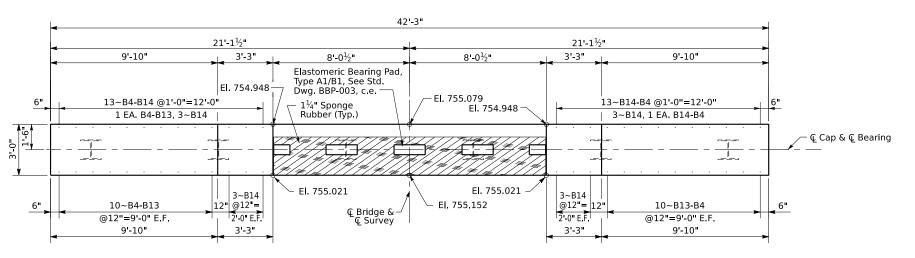
NOTE: Distances to bars shown are clear dimensions unless otherwise noted

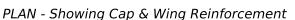
MATERIAL SPECIFICATIONS: Concrete, Class "A" Steel Reinforcement



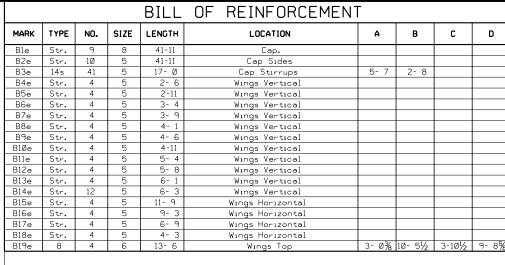
DETAIL ABOVE CAP

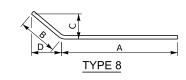






Station Ahead





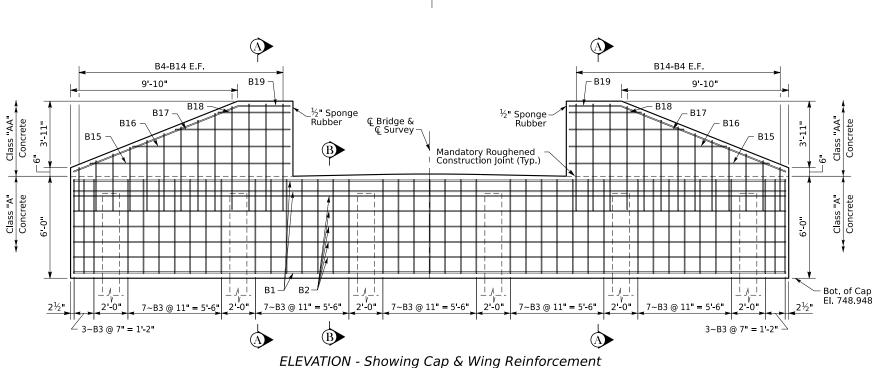


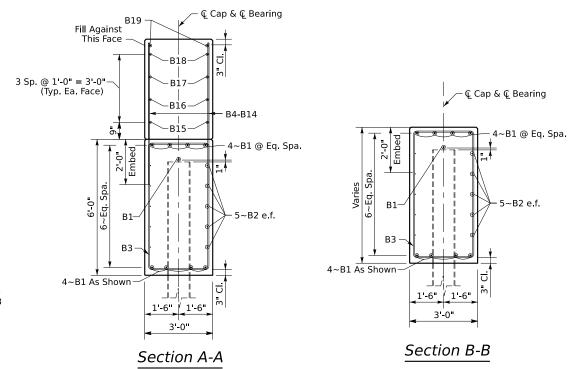
Pour Class "A" Concrete
 Erect beams and tension lateral rods

3) Pour Class "AA" Concrete with $\frac{1}{2}$ inch Sponge Rubber between the face of beams and wings. Place the designated $1\frac{1}{4}$ " inch Sponge Rubber underneath beams. Note: Elevations are given at the top of concrete.

Note: For pile location, see Foundation Layout.

Note: Place beams before backfilling.





COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION

Division of Structural Design

DATE: IUNE 2025 CHECKED BY DESIGNED BY: J. VAN ZEE K. EE DETAILED BY: M. BAWITHAWNG

INTEGRAL END BENT #2 Sturgeon Creek

OWSLEY Sturgeor Creek Rd 28995

PRECAST PRESTRESSED BOX BEAMS

General Notes

SPECIFICATIONS: All references to the standard Specifications are to the current edition of the Kentucky Department of Highways Standard Specifications for Road and Bridge Construction, with current supplemental specifications. All references to the AASHTO Specifications are to the current edition of the AASHTO LRFD Bridge Design Specifications, with interims.

DESIGN LOADS: Beam sections are designed for 1.25*HL93 (KYHL93) Live Load.

DESIGN LOAD DISTRIBUTION: Contrary to AASHTO LRFD Bridge Design Specifications, the design moment and shear distribution for all beams is 0.5 lanes.

FUTURE WEARING SURFACE: These beams are designed for a 15 PSF future wearing surface load.

SUBSTRUCTURE DESIGN LOADS: Unfactored design reaction forces per beam end. DC (kips): Beam, Slab (if applicable), and Type II railing dead loads.

DW (kips): Future wearing surface.

LL (kips): Beam Live Load reaction per lane x Design load distribution.

LL+I (kips): LL with Dynamic load allowance.

DESIGN DEFLECTIONS:

 Δd (in.): Sum of the downwards deflections caused by the design 5" deck, railing, and future wearing surface. (Positive Downwards) Δc (in.): Upwards midspan camber of the beam caused by prestressing minus the

downward deflection of the beam due to self weight. (Positive Upwards)

MATERIAL DESIGN SPECIFICATIONS:

for Steel Reinforcement for Prestressed Girder Concrete (Typ. U.N.O.) F'C = 7000 PSIF'CI = 5500 PSIfor Class "AA" Concrete F'C = 4000 PSIfor Prestressing Steel F'S = 270000 PSI

DESIGN LENGTH: Beam lengths shown in the Standards represent total beam length. Use the next greater designed section for non-Standard lengths.

CONSTRUCTION METHOD: Transferring bond stress to the concrete will not be allowed, nor releasing of end anchors until the concrete has attained a minimum compressive strength of F'Cl as shown by standard cylinders made and cured identically with the girders; attain F'C at or prior to 28 days. Apply an initial prestress force of 33817 lbs. per low relaxation strand. Beams with honeycomb of such extent as to affect the strength of resistance to deterioration will not be accepted. The allowance of .0005L (length) is made for shortening of beams due to shrinkage and elastic change. Furnish shop plans showing a detensioning plan by numbering, in sequence, the strand pattern.

PRESTRESSING STRANDS: Ensure prestressing strands to be $\frac{1}{2}$ " oversize (0.167 sq. in.) uncoated seven-wire stress relieved, low-relaxation strands conforming to AASHTO M 203, Grade 270. If an alternate strand arrangement or strand type is preferred by the Contractor, the designer that developed the original plans will provide the design and also revise the original plans to reflect the changes. These design and plan modifications will be done at the Contractor's expense

CORROSION INHIBITOR: Provide a corrosion inhibitor for B-type (non-composite) beams from the list of approved materials.

BEVELED EDGES: Bevel all exposed edges \(^3\)/4".

MicroStation v10.16.3.31

BEAM SEALER: For composite box beams (CB Beams), seal the full length of the exterior face of all exterior beams with the extent from the top of the beam to 1'-0" underneath the beam. For noncomposite box beams (B beams), seal all faces of all beams, except take care to ensure the grout pockets are not sealed. Use an approved silane sealer as specified by the Division of Structural Design.

DATE PLOTTED: 16-IUL-2025

REINFORCEMENT: Dimensions shown from the face of concrete to reinforcement are clear distances. Spacing of reinforcement is from center to center of reinforcement. All steel reinforcement is to be epoxy coated in accordance with Section 811.10 of the Specifications. Consider bars marked "C" to be a stirrup for purposes of bend diameters. Non-epoxy reinforcement may be used for fabrication purposes, only, provided that the steel is not used in the top 5 $\frac{1}{2}$ " of the beam and the location of the steel is indicated on the shop drawings.

FABRICATION: Beams shall not be fabricated more than 120 days before the deck is to be poured.

GROUT: Provide non-shrink grout for anchor dowels, shear keys, and tensioning rod block-outs conforming with Section 601.03.03 of the Specifications. When side by side superstructure is utilized, grouting will be completed after lateral tension rods have been fully tightened and before leveling devices have been removed. Include the cost of furnishing and placing grout in the price of beam.

RAILING SYSTEM TYPE II: Furnish this material per these specifications.

THEM	DESCRIPTION	MATERIAL SPECIFICATION	COATING SPECIFICATION
Post	W6x25	ASTM A36 or A572	A123
Channel	C7x9.8	ASTM A36 or A572	A123
Plate	½"x 7"	ASTM A36 or A572	A123
Tubing	8x4x0.1875	ASTM A500 or A501	A123
Bolts	5/ ₈ 11	ASTM A307	A153
Nuts	for 5/8"	ASTM A563 Cade A or better	A153
Washers	for 5%"	ASTM A563, Grade A or better	A153
Stud	11/4"	ASTM A108 (1045 C.D. Bar)	B633, Type II, Class 25
Ferrule	2½"x 5"	ASTM A108 (11L17 Steel)	B633, Type II, Class 25
Wire	3%"	ASTM A510 (1018 Steel)	B633, Type II, Class 25
Nut	for 1 1/4" Bolt	ASTM A108 (12L14 Steel)	B633, Type II, Class 25
Nut	for 1 $\frac{1}{4}$ " Stud	ASTM A325M	B633, Type H, Class 25
Washers	for 1 ½" Stud	ASTM A325M	B633, Type II, Class 25

Use the current edition of the references listed below with these standards.

STANDARD DRAWINGS

BBP-003 Elastomeric Bearing Pads BHS-007 Railing System Type II

BJE-001 Armored Edge & Neoprene Joints

RBR-001 Steel Beam Guardrail

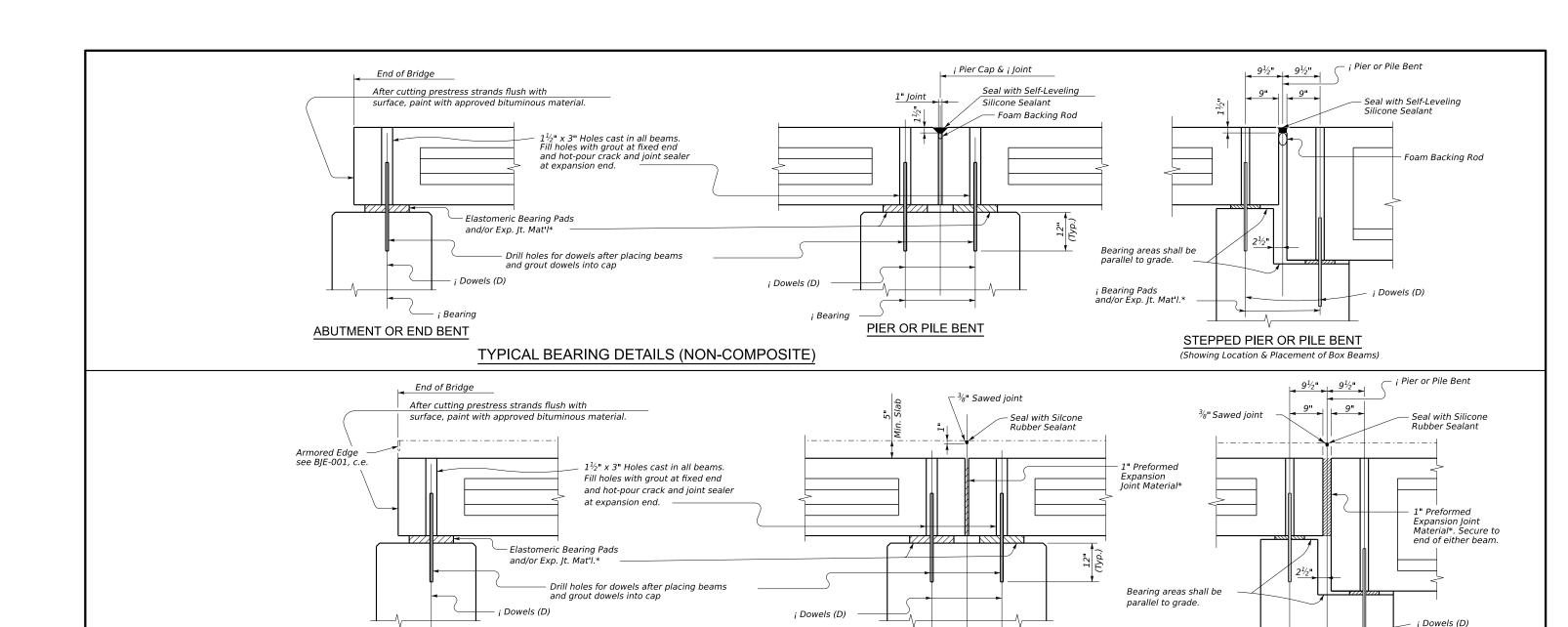
RBR-005 Guardrail Components

SPECIAL NOTES

for Corrosion Inhibitors

COMMONWEALTH OF KENTLICKY	REVISION	DATE	PREPARED BY	DATE: JUNE 2025	CHECKED BY	BOX BEAM GENERAL NOTES	ROUTE	ITEM NO.	COUNTY OF
			Division of	DESIGNED BY: J. VAN ZEE	V EE	DON DEAM CENERAL NOTES	Sturggon		OWSLEY
DEPARTMENT OF HIGHWAYS KENTÜCKY.			0. 1.D	DESIGNED BY: J. VAN ZEE	N. EE	CROSSING	Sturgeon	SHEET NO.	DRAWING NUMBER
Cuenci			Structural Design	DETAILED BY: M. BAWITHAWNG	J. VAN ZEE	Sturgeon Creek	Creek Rd	S8	28995

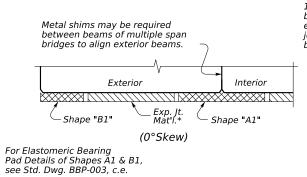
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TYPICAL BEARING DETAILS (COMPOSITE)

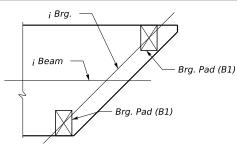
*Expansion Joint Material: AASHTO MÍ53 Type-I Sponge Rubber

** Preformed Neoprene Rubber Sheet: 50 Durometer AASHTO M251, Grade 3



Preformed Expansion Joint Material 1'-6" wide placed between Bearing Pads and beneath dowel pin holes to prevent the escape of mortar or joint sealer. Expansion Metal shims (8" x 12") may joint material* may be cemented to be required over bearing bottom of beam. pads on skewed bridges to insure uniform bearing. Exterior (Skewed Spans)

PIER OR PILE BENT



PAD PLACEMENT FOR SKEWS

Pads "B1" are to always be placed perpendicular to ; beam with center of pad over ; bearing

STEPPED PIER OR PILE BENT

(Showing Location & Placement of Box Beams)

SHOWING PADS FOR BEAM TYPES B27-B42 & CB27-CB42

Use $\frac{1}{2}$ " x 1'-6" preformed neoprene rubber sheet** for beam types B12-B21 & CB12-CB21 for bearing.

Provide metal shims conforming to ASTM A36 and galvanize in accordance with ASTM A123. As alternates, polymer, or elastomer shims may be used. Include the cost of furnishing and placing these shims in the price per beam.

GENERAL NOTES

10/25/2024 SUBMITTED ____

28995

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

REVISION DATE PLOTTED: 16-IUL-2025

Division of Structural Design

DATE: IUNE 2025 CHECKED BY DESIGNED BY DETAILED BY: J. VAN ZEE

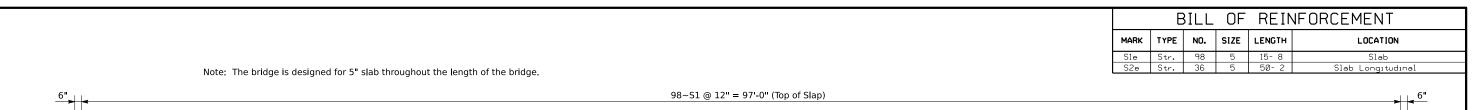
SEPIA 44-BOX BEAM BEARING DETAILS Creek Rd Sturgeon Creek

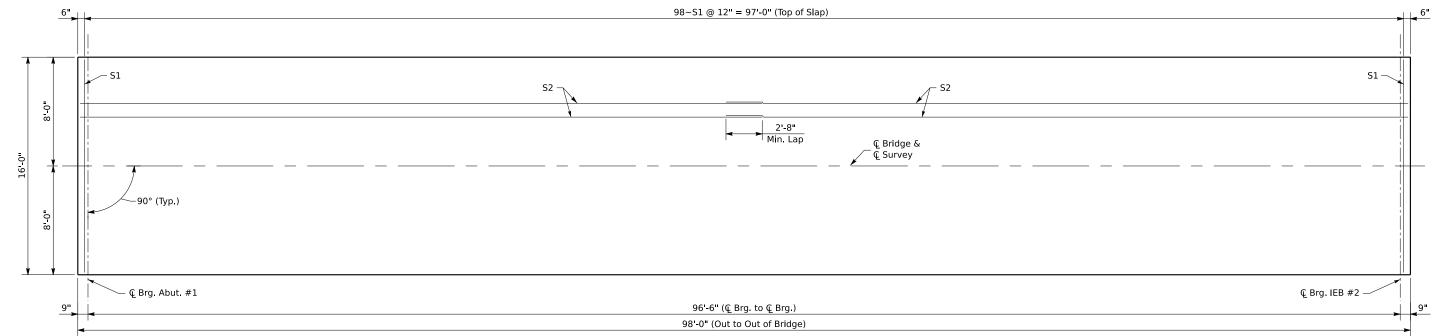
; Bearing Pads and/or Exp. Jt. Mat'l.*

> ROUTE OWSLEY Sturgeor

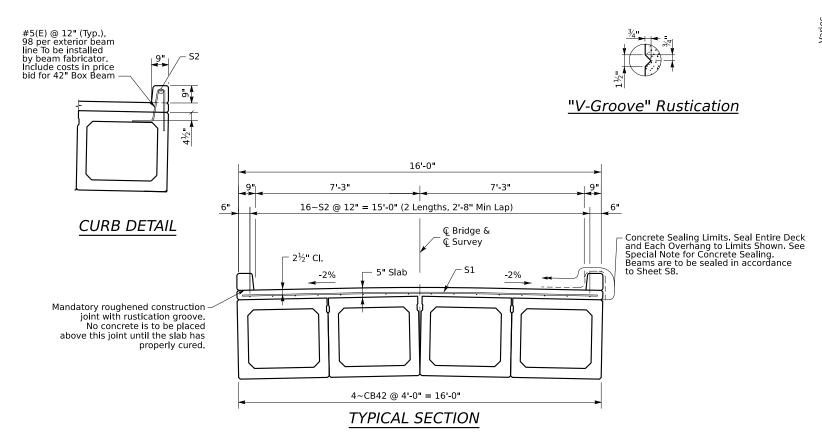
ABUTMENT OR END BENT

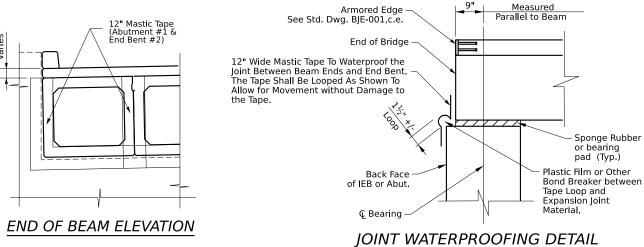
¡ Bearing





PLAN





GENERAL NOTES

MASTIC TAPE: Mastic Tape used to seal joints is to meet the requirements of ASTM C-877 Type I, II, or III. The joint is to be covered with 12-inch wide mastic tape. Prior to application, the joint surface shall be clean and free of dirt, debris, or deleterious material. Primer, if required by the tape mfgr., shall be applied for a minimum width of nine inches on each side of the joint.

Mastic Tape shall be either:

EZ-WRAP RUBBER by PRESS-SEAL GASKET CORPORATION, SEAL WRAP by MAR MAC MANUFACTURING CO. INC., CADILLOC by the UP RUBBER CO. INC. or approved equal

Mastic Tape shall cover the joint continuously unless otherwise shown in the plans. Mastic Tape shall be spliced by lapping a minimum of six inches and in accordance with the mfgrs. recommendations with the overlap running downhill.

All preformed expansion joint material, caulking, mastic tape, pipe sleeve and equipment and labor necessary to complete the joints are incidental to the square foot bid for Retaining Walls.

COMMONWEALTH OF KENTUCKY DEPARTMENT OF HIGHWAYS

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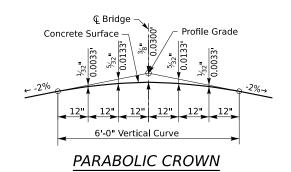
Division of Structural Design

DATE: IUNE 2025 CHECKED BY DESIGNED BY: J. VAN ZEE K. EE DETAILED BY: M. BAWITHAWNG J. VAN ZEE

SUPERSTRUCTURE Sturgeon Creek

ROUTE OWSLEY Sturgeor Creek Rd S10 28995

CONSTRUCTION ELEVATIONS									
LOCATION	LEFT GUTTER			CENTER LINE			RIGHT GUTTER		
	CONSTR. ELEV.	TOP OF BEAM	DIM. "X"	CONSTR.	TOP OF BEAM	DIM. "X"	CONSTR. ELEV.	TOP OF BEAM	DIM. "X"
Skew Line AA	761.363			761.494			761.363		
Skew Line BB	761.345			761.476			761.345		
Skew Line CC	759.009			759.139			759.009		
Skew Line DD	758.990			759.121			758.990		
Grid Line 1	761.250			761.381			761.250		
Grid Line 2	761.072			761.202			761.072		
Grld Line 3	760.891			761.022			760.891		
Grid Line 4	760.708			760.839			760.708		
Grid Line 5	760.521			760.652			760.521		-
Grid Line 6	760.332			760.462			760.332		
Grld Line 7	760.138			760.269			760.138		
Grid Line 8	759.940			760.071			759.940		
Grid Line 9	759.739			759.870			759.739		-
Grid Line 10	759.535			759.666			759.535		
Grld Line 11	759.328			759.459			759.328		
Grid Line 12	759.120			759.250			759.120		



NOTES FOR ELEVATIONS TAKEN ON PRESTRESSED CONCRETE BOX BEAMS

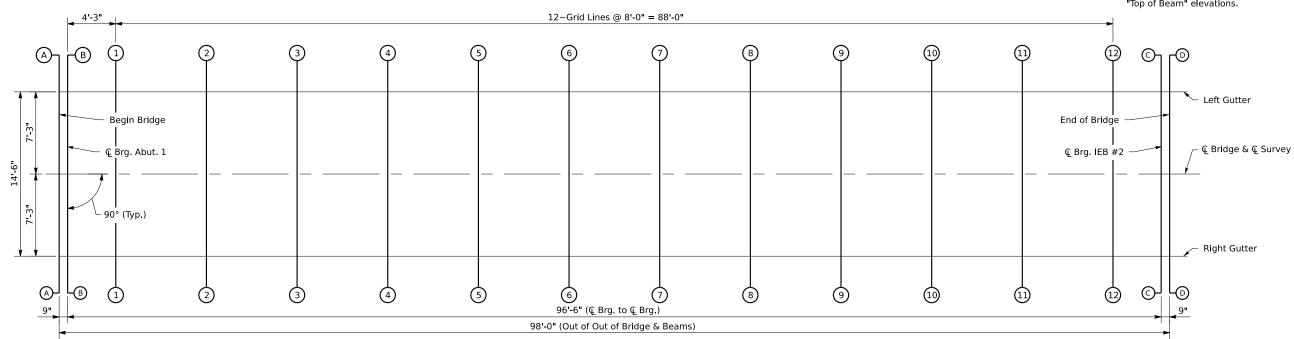
Take elevations on top of beam at points indicated after the beams have been laterally tensioned and grouted. The beam elevations are to be read to three decimal places and entered in tables under "Top of Beam" elevations.

Compute dimension "X" as follows: "Construction Elevation" minus "Top of Beam" elevation equals dimension "X". Construction Elevations include camber due to weight of the concrete slab and barrier. Measuring of dimension "X" gives the final check on beam tolerances for camber, beam damage, and errors in erection that produce reverse cambers, sags, and unsightly fascia beams.

The minimum allowable dimension "X" or slab thickness is $4\frac{3}{4}$ " (0.395') If any computed dimension "X" is less than that, adjustments may need to be made to the dimensions "X" on one or more grid lines.

For setting templates, measure dimension "X" above top of beams for top of template. Do not set template by elevations.

Temporary supports or shoring will not be permitted under the girders when pouring the concrete floor slab or when taking "Top of Beam" elevations.



GRID LAYOUT

